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*Roughness-Induced Vehicle Energy
Dissipation: Statistical Analysis and Scaling*

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Citation: Louhghalam, Arghavan, Mazdak Tootkaboni, and Franz-Josef Ulm. "Roughness-Induced Vehicle Energy Dissipation: Statistical Analysis and Scaling." *Journal of Engineering Mechanics* 141.11 (2015): 04015046.

As Published: [http://dx.doi.org/10.1061/\(asce\)em.1943-7889.0000944](http://dx.doi.org/10.1061/(asce)em.1943-7889.0000944)

Publisher: American Society of Civil Engineers (ASCE)

Persistent URL: <http://hdl.handle.net/1721.1/110690>

Version: Author's final manuscript: final author's manuscript post peer review, without publisher's formatting or copy editing

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1 **ROUGHNESS–INDUCED VEHICLE ENERGY DISSIPATION:**
2 **STATISTICAL ANALYSIS AND SCALING**

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4 **ABSTRACT**

5 The energy dissipated in vehicle’s suspension system due to road roughness affects rolling
6 resistance and the resulting fuel consumption and greenhouse gas emission. The key paramete-
7 ters driving this dissipation mechanism are identified via dimensional analysis. A mechanistic
8 model is proposed that relates vehicle dynamic properties and road roughness statistics to
9 vehicle dissipated energy and thus fuel consumption. Scaling relationship between the dissi-
10 pated energy and the most commonly used road roughness index, International Roughness
11 Index (IRI), is also established. It is shown that the dissipated energy scales with IRI
12 squared and scaling of dissipation with vehicle speed V depends on road waviness number

13 w in the form of V^{w-2} . The effect of marginal probability distribution of road roughness
14 profile on dissipated energy is examined. It is shown that while the marginal distribution of
15 road profile does not affect the identified scaling relationships, the multiplicative factor in
16 these relationships does change from one distribution to another. As an example of practical
17 application, the model is calibrated with the empirical HDM-4 model for different vehicle
18 classes.

19 **Keywords:** roughness-induced dissipation, pavement vehicle interaction, IRI, roughness
20 power spectral density, stationary stochastic process, translation process theory

21 INTRODUCTION

22 Pavement roughness affects rolling resistance (Beuving et al., 2004), and thus vehicle fuel
23 consumption. In fact, when a vehicle travels at constant speed on an uneven road surface,
24 the mechanical work dissipated in the vehicle’s suspension system is compensated by ve-
25 hicle engine power, resulting in excess fuel consumption. In addition to pavement texture
26 effects (Sandberg et al. (2011)) and viscoelastic dissipation in the pavement material (see
27 e.g., Pouget et al. (2011), Akbarian et al. (2012), Louhghalam et al. (2013), Louhghalam
28 et al. (2014b)), pavement roughness manifesting itself as surface unevenness with wave-
29 lengths above 50 mm (Flintsch et al., 2003), has been recognized as a main contributor
30 to Pavement Vehicle Interactions (PVI) affecting vehicle operating costs (VOC) (Zaabar
31 and Chatti (2010)). While the phenomenon is well known, the intricate links between road
32 roughness parameters, vehicle dynamic characteristics, and vehicle speed remain yet to be
33 established. The mechanistic model developed herein, aims at quantitatively assessing the
34 impact of these parameters on roughness-induced vehicle fuel consumption and the relating
35 greenhouse gas emission. Such models are in high demand for evaluating the environmental
36 footprint of pavement structures during their use-phase, contributing to the development of

37 a quantitative frameworks for pavement sustainable design and maintenance. The develop-
 38 ments presented in this paper aim at contributing to the growing field of mechanics-based
 39 quantitative engineering sustainability. In contrast to empirical approaches, the originality of
 40 the approach herein developed relies on a combination of a thermodynamic quantity (energy
 41 dissipation) with results from random vibration theory in order to identify scaling relations
 42 of roughness-induced vehicle energy dissipation.

43 To motivate the forthcoming developments, consider the classical two-degree-of-freedom
 44 (2-DOF) quarter-car model (Sayers (1995)) shown in Figure 1: a two-mass system in series
 45 composed of a tire (stiffness k_t) and a spring-dashpot parallel suspension unit (stiffness k_s
 46 and viscosity coefficient C_s). We are interested in the dissipation rate (δD) of mechanical
 47 work into heat form due to the relative motion, $\dot{z} = dz/dt$ (with z the relative displacement
 48 of sprung mass m_s with respect to the unsprung mass m_u) of the suspension unit. This
 49 dissipation depends on the vehicles dynamic properties (m_s, m_u, k_t, k_s, C_s), the vehicle speed
 50 V ; and parameters that quantify the pavement roughness. This roughness, ξ , is typically
 51 assessed by longitudinal profile data, and condensed, after Fourier transformation, into the
 52 power spectral density (PSD) of roughness which describes the distribution of roughness
 53 across various wavenumbers (Ω) in the form of $S_\xi(\Omega) = c\Omega^{-w}$, where c is the unevenness
 54 index and w is the waviness number (Dodds and Robson (1973), Robson (1979), Kropac and
 55 Mucka (2008)). We thus seek a relationship between the dissipation per distance traveled
 56 ($\delta\mathcal{E} = \delta D/V$) and these parameters; that is:

$$57 \quad \delta\mathcal{E} = \frac{C_s \dot{z}^2}{V} = f(m_s, m_u, k_t, k_s, C_s, V, c, \Omega_i) \quad (1)$$

58 It is useful to perform a dimensional analysis of Eq. (1) by considering an extended

59 base dimension system ($L_x L_z M T$) that considers, in addition to mass (M) and time (T),
60 two independent characteristic length dimensions, one for the driving direction (L_x), the
61 other for the vertical direction of vehicle motion (L_z). For instance, in this extended base
62 dimension system, the dissipation per lane mile traveled δD has dimension $[\delta \mathcal{E}] = [\delta D/V] =$
63 $[F_z][dz/dt][V]^{-1} = L_x^{-1} L_z^2 M T^{-2}$ (where F_z stands for the force in the dashpot); while the
64 speed has dimension $[V] = L_x T^{-1}$. Similarly, we obtain $[k_t] = [k_s] = M T^{-2}$, $[C_s] = M T^{-1}$,
65 $[\Omega_i] = L_x^{-1}$, whereas for the unevenness index $[c] = [S_\xi][\Omega^w] = L_x^{1-w} L_z^2$, since $[S_\xi(\Omega)] = L_x L_z^2$.
66 The exponent matrix of dimension reads for the problem thus defined:

$$\begin{array}{c|cccccccccc}
& [\delta \mathcal{E}] & [m_s] & [m_u] & [k_t] & [k_s] & [C_s] & [V] & [c] & [\Omega_i] \\
\hline
L_x & -1 & 0 & 0 & 0 & 0 & 0 & 1 & 1-w & -1 \\
L_z & 2 & 0 & 0 & 0 & 0 & 0 & 0 & 2 & 0 \\
M & 1 & 1 & 1 & 1 & 1 & 1 & 0 & 0 & 0 \\
T & -2 & 0 & 0 & -2 & -2 & -1 & -1 & 0 & 0
\end{array} \tag{2}$$

68 The rank of the matrix which characterizes the number of dimensionally independent param-
69 eters, is $k = 4$ independent of the value of w . This allows one, according to the PI-Theorem
70 (Buckingham (1914)), to reduce the dimensional problem defined by Eq. (1) to a dimen-
71 sionless relation of the form:

$$\frac{\delta \mathcal{E}}{c C_s V^{w-2} \omega_s^{3-w}} = F \left(\gamma = \frac{m_u}{m_s}, \beta = \frac{\omega_u}{\omega_s}, \zeta = \frac{C_s}{2 m_s \omega_s}, \bar{\omega}_i = \frac{\omega_i}{\omega_s} \right) \tag{3}$$

73 where $\omega_u = \sqrt{k_t/m_u}$ and $\omega_s = \sqrt{k_s/m_s}$ are the natural frequency of respectively the un-
74 sprung and the sprung masses, whereas $\omega_i = V \Omega_i$ stands for the angular frequencies. The
75 dimensional analysis is able to isolate –on the left-hand-side of Eq. (3)– the impact of

76 pavement roughness (captured by the unevenness index c and the waviness number w) on
77 the dissipation, from the dimensionless dynamic vehicle properties on the right-hand-side,
78 namely mass-ratio $\gamma = m_u/m_s$, natural frequency ratio $\beta = \omega_u/\omega_s$, and damping ratio
79 $\zeta = C_s/(2m\omega_s)$. Specifically, it reveals that the energy dissipation scales with the vehicle
80 speed as $\delta\mathcal{E} \propto V^{w-2}$. That is, for waviness numbers $w > 2$, the dissipated energy increases
81 with the vehicle speed, and for values $w < 2$, it is the inverse.

82 With the problem thus defined, the focus of the rest of this paper is to quantify by means
83 of a mechanistic modeling the relationship between vehicle properties and road roughness
84 statistics, and the dimensionless roughness-induced energy dissipation.

85 **ROUGHNESS-INDUCED DISSIPATION**

86 Since road roughness ξ is random, the suspension motion and consequently energy dissi-
87 pation in Eq. (1) are stochastic quantities. Modeling road roughness and suspension motion
88 as stochastic processes defined in space and time, Eq. (1) is rewritten in the form:

$$89 \quad \mathbb{E}[\delta\mathcal{E}] = \frac{C_s}{V} \mathbb{E}[\dot{z}^2] \quad (4)$$

90 where $\mathbb{E}[\cdot]$ denotes the operation of mathematical expectation. The mean-square of sus-
91 pension motion can be determined in terms of properties of the stochastic input, namely
92 roughness profile, using random vibrations theory. In what follows a brief review of the
93 elements of this theory used in our model development is provided. Readers interested in
94 more details are referred to classical textbooks on the subject (see e.g. Crandall and Mark
95 (1963) and Lutes and Sarkani (1997)).

Elements of Random Vibration Theory

Stochastic Processes, Definition and Properties

Function $\xi(t)$ of an independent variable t is a random process, if $\xi(t_i)$ is a random variable for any value of t_i . Independent variable t can represent time or space for temporally or spatially varying stochastic processes. Moments of a stochastic process provide a great deal of information about its characteristics. The first moment is the mean $\mu_\xi(t) = E[\xi(t)]$ and the second moment is the autocovariance function:

$$K_\xi(s, t) = E[(\xi(t) - \mu_\xi(t))(\xi(s) - \mu_\xi(s))] \quad (5)$$

The autocorrelation function of a stochastic process, which is identical to the autocovariance function for zero-mean processes, is defined as:

$$R_\xi(s, t) = E[\xi(t)\xi(s)] \quad (6)$$

A stationary random process has properties that are independent of the absolute time values; i.e. for the case of first two moments (so-called weakly stationary), the mean value does not depend on time ($\mu_\xi(t) = \mu$), and the autocorrelation function depends only on the time difference or lag ($R_\xi(s, t) = R_\xi(\tau = t - s)$). For a zero-mean stationary process the Wiener-Khintchine theorem (Khintchine (1934), Champeney (1987)) states that autocorrelation function $R_\xi(\tau)$ and power spectral density function $S_\xi(\omega)$ are Fourier transform

113 pairs:

$$114 \quad S_{\xi}(\omega) = \frac{1}{2\pi} \int_{-\infty}^{\infty} R_{\xi}(\tau) \exp(-i\omega\tau) d\tau \quad (7)$$

$$115 \quad R_{\xi}(\tau) = \int_{-\infty}^{\infty} S_{\xi}(\omega) \exp(i\omega\tau) d\omega \quad (8)$$

116 where ω is the angular frequency. The power spectral density (PSD) of a stationary process
 117 which is truncated at $\pm T/2$ can also be expressed in terms of the Fourier transform of that
 118 process:

$$119 \quad S_{\xi}(\omega) = \lim_{T \rightarrow \infty} \frac{2\pi}{T} \mathbb{E} \left[\left| \widehat{\xi}_T(\omega) \right|^2 \right] \quad (9)$$

120 with $\widehat{\cdot}$ denoting Fourier transform. It can be shown that for any stochastic process $\xi(t)$,
 121 PSD function $S_{\xi}(\omega)$ is positive, real and even; hence it can also be specified as a one-sided
 122 function over only positive frequencies. Of special interest is the case where the time lag is
 123 $\tau = 0$ in Eq. (8), since this gives the mean-square of $\xi(t)$ as the area under its PSD:

$$124 \quad \mathbb{E} [\xi^2(t)] = R_{\xi}(0) = \int_0^{\infty} S_{\xi}(\omega) d\omega \quad (10)$$

125 The stochastic process $\xi(t)$ is Gaussian (normal), if the random variables $\{\xi(t_i)\}_{i=1}^n$
 126 are jointly Gaussian for any $n \in \mathbb{N}$ and all values of t_i . A stationary Gaussian process is
 127 completely characterized by its mean μ and autocorrelation function $R_{\xi}(\tau)$.

128 *Response of a Linear Dynamical System to Random Excitations*

129 Once the input excitation $\xi(t)$ to a linear system is decomposed into its harmonics via
 130 Fourier transformation, the steady-state response in frequency domain $\widehat{z}(\omega)$ can be expressed
 131 as:

$$132 \quad \widehat{z}(\omega) = H_z(\omega) \widehat{\xi}(\omega) \quad (11)$$

133 where $H_z(\omega)$ is the frequency response function (FRS) defined as the ratio of input excitation
 134 $\xi(t)$ to output of interest $z(t)$ when input is the pure harmonic (i.e. when $\xi(t) = \exp(i\omega t)$).
 135 Frequency response function for derivatives of response is readily obtained from the fre-
 136 quency response function of the original response using the properties of Fourier transform
 137 of derivatives (i.e. $\widehat{dx(t)/dt} = i\omega\widehat{x(\omega)}$):

$$138 \quad H_{\dot{z}}(\omega) = i\omega H_z(\omega) \quad (12)$$

139 Once FRS is known, the PSD of response can be related to the PSD of input excitation via:

$$140 \quad S_z(\omega) = |H_z(\omega)|^2 S_\xi(\omega) \quad (13)$$

141 **Roughness-Induced Dissipation in the Quarter-Car**

142 For the 2-DOF quarter-car system in Figure 1 subjected to a displacement excitation
 143 $\xi(t)$ the equations of motion can be expressed in terms of the dimensionless parameters in
 144 Eq. (3) in the form:

$$145 \quad \begin{bmatrix} 1 & 0 \\ 1 & \gamma \end{bmatrix} \begin{bmatrix} \ddot{y}_s \\ \ddot{y}_u \end{bmatrix} + 2\omega_s\zeta \begin{bmatrix} 1 & -1 \\ 0 & 0 \end{bmatrix} \begin{bmatrix} \dot{y}_s \\ \dot{y}_u \end{bmatrix} + \omega_s^2 \begin{bmatrix} 1 & -1 \\ 0 & \gamma\beta^2 \end{bmatrix} \begin{bmatrix} y_s \\ y_u \end{bmatrix} = \begin{bmatrix} 0 \\ 1 \end{bmatrix} \gamma\beta^2\omega_s^2\xi(t) \quad (14)$$

146 with y_s and y_u denoting the displacement of sprung and unsprung masses, respectively. In
 147 the presence of a pure harmonic input $\xi(t) = \exp(i\omega t)$ these displacement responses are
 148 expressed as $y_s = H_{y_s}(\omega) \exp(i\omega t)$ and $y_u = H_{y_u}(\omega) \exp(i\omega t)$. The frequency response

149 functions are obtained by substituting these expressions in Eq. (14) and are of the form:

$$150 \quad \begin{bmatrix} H_{y_s}(\omega) \\ H_{y_u}(\omega) \end{bmatrix} = \left(\begin{bmatrix} -\bar{\omega}^2 + \kappa & -\kappa \\ -\bar{\omega}^2 & -\bar{\omega}^2\gamma + \gamma\beta^2 \end{bmatrix} \right)^{-1} \begin{bmatrix} 0 \\ \gamma\beta^2 \end{bmatrix} \quad (15)$$

151 with $\kappa = 2i\bar{\omega}\zeta + 1$. The frequency response function of interest herein relates the relative
 152 displacement between the two masses $z = y_s - y_u$ to the input excitation in frequency domain
 153 (i.e. $\hat{z}(\omega) = H_z(\omega) \hat{\xi}(\omega)$), and is obtained from:

$$154 \quad H_z(\omega) = H_{y_s}(\omega) - H_{y_u}(\omega) = \frac{-\bar{\omega}^2\gamma\beta^2}{(\bar{\omega}^2 - \kappa)(\bar{\omega}^2\gamma - \gamma\beta^2) - \bar{\omega}^2\kappa} \quad (16)$$

155 Using Eqs. (10), (11) and (12) the mean-square of suspension motion is expressed in terms
 156 of the frequency response function $H_z(\omega)$ and power spectral density of roughness $S_\xi(\omega)$:

$$157 \quad \mathbb{E}[\dot{z}^2] = \int_0^\infty S_z(\omega) d\omega = \int_0^\infty \omega^2 |H_z(\omega)|^2 S_\xi(\omega) d\omega \quad (17)$$

158 For a vehicle traveling with constant speed V the PSD of roughness $S_\xi(\omega)$ in function of the
 159 angular frequency relates to the PSD of roughness in function of the wave number $\Omega = \omega/V$
 160 through $S_\xi(\Omega) = VS_\xi(\omega)$. The expected value of dissipation in Eq. (4) thus reads:

$$161 \quad \mathbb{E}[\delta\mathcal{E}] = C_s V^{w-2} c \int_0^\infty \omega^{2-w} |H_z(\omega)|^2 d\omega \quad (18)$$

162 or in the dimensionless functional form Eq. (3) expressed in terms of road roughness variables
 163 (c and w) as well as vehicle parameters (γ , β , and ζ):

$$164 \quad \Pi = \frac{\mathbb{E}[\delta\mathcal{E}]}{m_s \omega_s^{4-w} V^{w-2} c} = 2\zeta \int_0^\infty \bar{\omega}^{2-w} |H_z(\bar{\omega})|^2 d\bar{\omega} = F(\gamma, \beta, \zeta, w) \quad (19)$$

165 The result of the above analysis has two main practical applications. First, it provides a
166 means to understand how roughness-induced dissipation scales with various vehicle and road
167 parameters. Second, one can relate the above mechanistic model with mechanical-empirical
168 models developed to estimate vehicle fuel consumption, such as calibrated HDM-4 model
169 (Chatti and Zaabar, 2012). The insight gained from the mechanistic approach can be used
170 to advance such models.

171 **Relation with IRI**

172 To achieve the above goals one important step is to establish a relationship between
173 the dissipated energy and frequently used roughness metrics. It is common practice to
174 capture road roughness through a single roughness index, such as the Average Rectified
175 Slope (ARS), which is the accumulated suspension motion divided by the distance traveled,
176 i.e. $ARS = (VL)^{-1} \int_0^L |\dot{z}| dx$ (Sayers et al. (1986), Johannesson and Rychlik (2012)). For
177 a specific quarter-car, the golden-car with properties shown in Table 1 traveling at a speed
178 of $V_0 = 80$ km/h, ARS corresponds to the International Roughness Index (IRI). Here we
179 assume that the road profile can be modeled via a zero-mean Gaussian process (Dodds and
180 Robson (1973), ISO-8608 (1995), Sun et al. (2001)) – we comment later on how the results
181 are affected if the road profile consists of bumps and valleys that cannot be captured by
182 the “light” tails of a Gaussian distribution. Assuming a Gaussian marginal distribution for
183 road profile, the absolute value of golden-car suspension motion $|\dot{z}|$ follows a folded normal
184 distribution with mean $\sqrt{2E[\dot{z}^2]}/\pi$ (Leone et al., 1961). The expected value of IRI thus
185 reads:

$$186 \quad E[\text{IRI}] = \frac{1}{V_0} \sqrt{\frac{2}{\pi}} \left[\int_0^\infty \omega^2 |H_{z-GC}(\omega)|^2 S_\xi(\omega) d\omega \right]^{1/2} \quad (20)$$

187 with subscript GC denoting that the quantity relates to the properties of the golden-car.
 188 The above equation can be expressed in terms of the PSD parameters:

$$189 \quad E[\text{IRI}] = \left[\frac{2c}{\pi V_0^{3-w}} \int_0^\infty \omega^{2-w} |H_{z-GC}(\omega)|^2 d\omega \right]^{1/2} \quad (21)$$

190 We note that IRI depends both on the golden-car dynamic properties (through frequency
 191 response function $H_{z-GC}(\omega)$), and road roughness characteristics via roughness PSD param-
 192 eters c and w . The above relation can be written in terms of the dimensionless variables
 193 defined in (3):

$$194 \quad E[\text{IRI}] = \left[\frac{\omega_{s-GC}^{3-w}}{V_0^{3-w}} \frac{\Pi_{GC}}{\pi \zeta_{GC}} c \right]^{1/2} = \alpha \sqrt{c} \quad (22)$$

195 The dimensionless dissipation of the golden-car, $\Pi_{GC} = \Pi(\gamma_{GC}, \beta_{GC}, \zeta_{GC})$ is evaluated from
 196 Eqs. (16) and (19) using the values given in Table 1. It only depends on the waviness number
 197 w , which typically varies between 1.5 and 3 (Kropac and Mucka (2004)). Specifically, for
 198 $w = 2$ as suggested by the International Standard Organization (ISO) (ISO-8608, 1995), the
 199 following relation between unevenness index c and IRI is obtained:

$$200 \quad E[\text{IRI}] = \sqrt{\frac{\omega_{s-GC}}{V_0} \frac{\Pi_{GC}}{\pi \zeta_{GC}} c} \quad (23)$$

201 which agrees with the expression $E[\text{IRI}] = 2.21 \sqrt{c}$ reported by Kropac and Mucka (2004) and
 202 Johannesson and Rychlik (2012). Finally, eliminating the unevenness index c between Eqs.
 203 (19) and (22), the expected value of dissipated energy is obtained in function of IRI and
 204 waviness number w :

$$205 \quad E[\delta \mathcal{E}] = k_d^2 m_s \omega_{s-GC} \zeta_{GC} V_0 \left(\frac{V_0}{V} \right)^{2-w} \left(\frac{\omega_s}{\omega_{s-GC}} \right)^{4-w} \frac{\Pi}{\Pi_{GC}} E[\text{IRI}]^2 \quad (24)$$

206 where $k_d = \sqrt{2}/\kappa$ with $\kappa = E[|\dot{z}|]/\sqrt{E[\dot{z}^2]}$. The coefficient k_d in the above equation depends
 207 on the marginal distribution of the suspension motion process and is equal to π when the
 208 road profile (and consequently the suspension motion) follows a Gaussian distribution. Other
 209 forms of distributions will be addressed later on.

210 **Scaling of Energy Dissipation**

211 We are interested in the scaling of energy dissipation with different vehicle and road
 212 properties.

213 **Scaling with road condition:** The expected value of dissipation is proportional to $E[IRI]^2$.
 214 For a specific vehicle (i.e. constant values of β, γ and ζ) the ratio of dimensionless dissipation
 215 Π/Π_{GC} only depends on w , and decreases as w increases. Figures 2 and 3 show the variation
 216 of this dimensionless ratio as a function of w for different values of β, γ . The dissipation
 217 also scales with vehicle speed as V^{w-2} which indicates that dissipation increases with speed
 218 if $w > 2$, decreases with speed if $w < 2$ and is independent of speed when $w = 2$.

219 **Scaling with vehicle properties:** For fixed road condition (i.e. fixed values of IRI and
 220 w), the dissipation scales according to Eq. (24), with vehicle sprung mass m_s and the
 221 corresponding natural frequency as ω_s^{4-w} . Our parametric studies show that the roughness-
 222 induced dissipation does not change significantly with variation in the dimensionless damping
 223 ratio ζ , and therefore it is disregarded in the analysis. The ratio of dimensionless dissipation
 224 Π/Π_{GC} in functions of the two vehicle specific invariants β and γ shown in Figure 4 reveals
 225 that for a specific road condition, the dimensionless dissipation increases with both β and γ .

226 **Special case $w = 2$:** For the special case of $w = 2$ one can express the dimensionless
227 dissipation in function of dimensionless invariants γ and β :

$$228 \quad \Pi = \frac{\pi\gamma\beta^2}{2} \quad (25)$$

229 Therefore the roughness-induced dissipation reduces to:

$$230 \quad \text{E}[\delta\mathcal{E}] = m_s\omega_s^2c\frac{\pi\gamma\beta^2}{2} = \frac{\pi c}{2}k_t \quad (26)$$

231 That is, for a specific road roughness, the dissipation only depends on tire stiffness, k_t .

232 **IMPACT OF MARGINAL PROBABILITY DENSITY FUNCTION: NON-GAUSSIAN** 233 **BUMPS AND VALLEYS**

234 Here we discuss how the results presented in previous section are affected if the road profile
235 data exhibits bumps and valleys that are not captured by the “light” tails of a Gaussian
236 distribution. In fact, there is evidence that non-Gaussian distributions with heavier tails
237 are better suited for modeling the frequency of observed values in road elevation profile
238 data, especially when the phenomenon to be examined is analyzed for longer sections of
239 the road profile (Bruscella et al. (1999), Steinwolf and Connon (2005), Johannesson and
240 Rychlik (2013)). For example, Bruscella et al. (1999) analyzed several hundred kilometers of
241 Victorian (Australia) road profile data, and observed that the normalized histogram of the
242 elevation profile has heavier tails compared to that of a Gaussian distribution. Figure 5 shows
243 the empirical probability distribution function (PDF) associated with the data reported in
244 this study against a Gaussian distribution with the same mean and variance in both linear
245 and logarithmic scales. Deviation from Gaussian assumption is evident indicating that the
246 road profile needs to be modeled as a non-Gaussian stochastic process. In fact using a

247 Gaussian distribution to model marginal distribution of the elevation profile in such cases
248 results in the loss of statistically uncommon events and extreme values (e.g. elevations
249 exceeding ± 3 standard deviation pertaining to, for example, faulting).

250 A natural way to relax the Gaussian assumption and enrich the modeling process by
251 incorporating distributions of the type shown in Figure 5 where the lack of shoulders and
252 heavy tails are the main attributes, is to use probability distributions with higher kurtosis.
253 Kurtosis, defined as the ratio of fourth central moment to the square of the variance ($\beta_2 =$
254 μ_4/μ_2^2 , with μ_n the n^{th} central moment), is a measure of tail weight and peakedness in
255 a distribution (with higher kurtosis representing heavier tails and more peakedness). It
256 represents a movement of probability mass that does not affect the variance. It is thus
257 instructive to compare the empirical probability density function of road profile data to
258 distributions which look similar to the Gaussian distribution but have heavier tails and
259 higher peaks, and choose a distribution that best fits the profile data. Figure 6 illustrates
260 PDF of the road profile examined by Bruscella et al. (1999) along with Gaussian distribution
261 ($\beta_2 = 3$) and three non-Gaussian but symmetric distributions, i.e. logistic distribution,
262 hyperbolic secant distribution and Laplace distribution, each having kurtosis equal to 4.2,
263 5 and 6 respectively. It is observed that PDF of the road profile matches closely a Laplace
264 distribution which has the heaviest tails among the three distributions.

265 Adopting a non-Gaussian distribution to describe the road profile data the scaling re-
266 lationships previously derived need to be revisited. This is achieved by using a simulation
267 framework that allows the generation of realizations from a non-Gaussian random process
268 given its power spectral density (c and w in our case) and marginal distribution. Once such
269 realizations of the road profile are available they are fed into the golden-car model to ob-
270 tain realizations of IRI. The functional relationship between c and IRI is then numerically

271 determined by averaging over multiple runs, with each run simulating the dynamics of the
 272 golden-car run over a single realization of the road profile.

273 **Translation Process Theory**

274 Realizations of the road profile are generated based on the translation process theory
 275 introduced by Nataf (1962) and later developed by Grigoriu (1984), Liu and Der Kiureghian
 276 (1986) and Grigoriu (1998). A nonlinear transformation of the form:

$$277 \quad Y(s) = F_Y^{-1}(\Phi(X(s))) \quad (27)$$

278 with $X(s)$ a standard stationary Gaussian process, is used to model a stationary non-
 279 Gaussian process $Y(s)$. Herein, $F_Y(y)$ is the (target) marginal cumulative distribution func-
 280 tion of the process $Y(s)$, and Φ represents the standard Gaussian cumulative distribution
 281 function. It can be shown that the autocorrelation function of the resulting non-Gaussian
 282 process $R_Y(\tau)$ is related to the autocorrelation function of the underlying Gaussian process
 283 $R(\tau)$ by (Grigoriu (1998)):

$$284 \quad R_Y(\tau) = \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} F_Y^{-1}(\Phi(x_1)) F_Y^{-1}(\Phi(x_2)) \phi(x_1, x_2, \rho(\tau)) dx_1 dx_2 \quad (28)$$

285 with $\rho(\tau) = R(\tau)/\sigma^2$ and ϕ the bivariate Gaussian probability density function. Generating
 286 realizations of the non-Gaussian process then boils down to: (i) finding the autocorrelation
 287 function $R(\tau)$ of the Gaussian process given the target autocorrelation function or PSD
 288 (note these two are directly related). This is achieved by numerically inverting Eq. (28)
 289 provided the target marginal CDF and autocorrelation functions are “compatible”, or by
 290 means of iterative schemes that converge toward the best match for the autocorrelation
 291 function or the associated PSD (see Grigoriu (1998) or Shields et al. (2011)); (ii) generating

292 samples of the Gaussian process using one of the available techniques (see e.g. Shinozuka
 293 and Deodatis (1991) and Grigoriu (1993)); and (iii) using the nonlinear transformation Eq.
 294 (27) to generate samples of the non-Gaussian process.

295 **Impact on Scaling Relations**

296 A total of 5,000 road profile realizations were generated from a stochastic process with
 297 Laplace marginal distribution and roughness PSD function $S_\xi = c\Omega^{-w}$. Two sample profiles
 298 are illustrated in Figure 7 along with their Gaussian counterparts. The profiles with marginal
 299 Laplace distributions have more observable bumps and extremes compared to the ones with
 300 Gaussian distribution. The response, i.e. suspension motion of the golden-car, \dot{z} , due to road
 301 roughness is then evaluated by solving the equations of motion Eq. (14) for each of these
 302 realizations. The average IRI of simulated profiles is evaluated and plotted in function of c in
 303 Figure 8 for a wide range of unevenness index c that covers the range of IRI values in practice.
 304 While scaling with IRI of the unevenness index remains constant, the multiplicative factor
 305 α in the functional relationship (22) changes. We also included the $c - \text{IRI}$ curves obtained
 306 by approximating the marginal PDF of the suspension motion process by the same family of
 307 distributions as the one used in describing the road profile elevation. The curve associated
 308 with logistic distribution is, for example, obtained assuming the following distribution of \dot{z} :

$$309 \quad f(\dot{z}) = \frac{1}{4s} \operatorname{sech}^2\left(\frac{\dot{z} - \mu_{\dot{z}}}{2s}\right) \quad (29)$$

310 where $\mu_{\dot{z}}$ is the mean value and $s = \sqrt{3}\sigma_{\dot{z}}/\pi$ with $\sigma_{\dot{z}}$ standard deviation of \dot{z} . The absolute
 311 value of response $Y = |\dot{z}|$ has then a folded logistic distribution with the following CDF:

$$312 \quad F_Y(y) = \frac{1}{1 + \exp\left(-\frac{y - \mu_{\dot{z}}}{s}\right)} + \frac{1}{1 + \exp\left(-\frac{y + \mu_{\dot{z}}}{s}\right)} - 1 \quad (30)$$

313 For the special case of zero-mean \dot{z} , one can readily show that $E [|\dot{z}|] = 2s \ln 2$. Eqs. (21)
 314 and (22) can thus be rewritten as follows:

$$315 \quad E[\text{IRI}] = \frac{2 \ln 2}{\pi} \left[\frac{3c}{V_0^{3-w}} \int_0^\infty \omega^{2-w} |H_z(\omega)|^2 d\omega \right]^{1/2} = \frac{\ln 2}{\pi} \left[c \frac{6\omega_s^{3-w} \Pi_{GC}}{V_0^{3-w} \zeta_{GC}} \right]^{1/2} \quad (31)$$

316 Table 2 summarizes the result of similar calculations for different distributions of the
 317 marginal PDF of the suspension motion. Numbers associated with a Gaussian marginal
 318 PDF are also included for the sake of comparison. As shown in Figure 8 the hyperbolic
 319 secant distribution provides the best approximation for the functional relationship between
 320 unevenness index c and IRI. This can be explained by carefully examining the marginal
 321 distribution of suspension response. In fact, Figures 9 and 10 depict this distribution for
 322 waviness number $w = 2.5$ and different values of unevenness index c plotted against the
 323 associated Gaussian, Laplace, hyperbolic secant and logistic distributions. It is observed
 324 that the marginal PDF of response has heavier tails than that of a Gaussian distribution.
 325 This is manifested in a higher kurtosis value of 4.85 as compared to that of a Gaussian
 326 distribution, 3. Comparing the marginal PDF of \dot{z} to three different non-Gaussian PDFs,
 327 one also observes that the marginal distribution of suspension motion process is very close
 328 to hyperbolic secant distribution.

329 **APPLICATION: HANDSHAKE WITH HDM-4 MODEL**

330 The framework developed herein relates road surface characteristics and dynamic prop-
 331 erties of a quarter-vehicle to the roughness-induced dissipation, and thus fuel consumption.
 332 In practice, however, vehicle dynamics is far more complex than the simplified quarter-car
 333 model. In addition, measuring all dynamic properties of vehicle with a reasonable accuracy
 334 may not be always feasible. For instance, while it is possible to measure the inertial proper-

335 ties (sprung and unsprung masses) accurately, the total stiffness involved in different parts
336 of a vehicle is more complicated than stiffness of suspension and tire and may be very hard
337 to measure. In the absence of such detailed measurements empirical models such as the
338 HDM-4 model (Zaabar and Chatti (2010)) can be used to calibrate the proposed roughness-
339 induced mechanistic model. In such a calibration, the stiffness properties for different classes
340 of vehicles, together with the road waviness number as an additional adjustable parameter
341 are estimated (Louhghalam et al. (2014a)). The HDM-4 model reports the variation of ex-
342 cess fuel consumption due to change in IRI at different vehicle speeds and for five classes of
343 vehicle: medium car, SUV, van, light truck and heavy truck.

344 To calibrate the mechanistic model, the dissipated energy is first converted to fuel con-
345 sumption using engine efficiency coefficient (ξ_b in mL/kW/s). The calibration parameters,
346 i.e. stiffness properties of each vehicle class and a single road waviness number, are then
347 determined by minimizing the difference between two model predictions of change in fuel
348 consumption. Calibration is performed for practical ranges of IRI and vehicle speed, corre-
349 sponding to the field measurements in Chatti and Zaabar (2012). A detailed description of
350 the calibration procedure is explained in Louhghalam et al. (2014a), where the marginal dis-
351 tribution of road profile was assumed to be Gaussian (see Table 3 for a summary of results).
352 The results of our calibrated model are illustrated in Figure 11 for vehicle speeds 70 and 100
353 km/h and compared with the predictions of calibrated HDM-4 model (Chatti and Zaabar
354 (2012)). The plots show the change in total fuel consumption in function of IRI for five vehicle
355 classes. In contrast to the HDM-4 model, where fuel consumption is linearly related to IRI,
356 the developed mechanistic model establishes a quadratic relation between energy dissipation
357 and IRI. It is worth noting that the HDM-4 model for estimating roughness-induced fuel
358 consumption is an empirical model where a functional relationship, presumed to be varying

359 linearly with IRI, is fitted to the experimental measurements. Hence the scaling in IRI is as-
360 sumed a priori and is not the result of dynamic analysis of road roughness-vehicle interaction.
361 The calibration parameters can also be determined if the Gaussian assumption for profiles is
362 relaxed. Table 3 also shows the stiffness parameters for the three non-Gaussian distributions
363 discussed before. The waviness number $w = 2.4117$ is the same for all distributions studied,
364 which agrees well with the results of statistical analysis of the Long-Term Pavement Per-
365 formance program of the US Federal Highway Administration (FHWA) reported by Kropac
366 and Mucka (2008), exhibiting a mode at around $w = 2.5$.

367 **CONCLUDING REMARKS**

368 The mechanistic model developed in this paper quantifies the impact of road roughness
369 characteristics on vehicle fuel consumption as one of the sources of energy dissipation related
370 to rolling resistance. Such models are necessary for assessing the environmental footprint
371 of pavement structures during their use phase, thus contributing to the emerging quanti-
372 tative framework of engineering sustainability. The unique feature of this model is that it
373 integrates the uncertainty in pavement profiles into a thermodynamic quantity (energy dissi-
374 pation) using random vibration techniques. This provides a means to identify the governing
375 parameters that drive roughness-induced dissipation and related excess fuel consumption.

376 The results of our analysis establish the relationship between the statistical characteris-
377 tics of road profile and vehicle dynamic properties and energy dissipation. The scaling of
378 dissipation with IRI proposed by the mechanistic model (i.e. $\delta\mathcal{E} \propto \text{IRI}^2$) is different from
379 the linear scaling of dissipation with IRI reported by empirical models such as HDM-4. In
380 our mechanistic model the road roughness is represented by two independent parameters,
381 IRI and w . This is in contrast with empirical models in which only IRI is normally used to
382 represent the road surface condition.

383 Furthermore, for a specific vehicle, scaling of the dissipated energy with speed varies
384 with the waviness number as $E[\delta\mathcal{E}] \propto V^{w-2}$. This implies that the dissipation increases
385 with speed for $w > 2$, and for $w < 2$ it is the inverse. In return, since the waviness number
386 of pavements varies in the range $w = 2.5 \pm 0.5$, the variation of speed does not change
387 the roughness-induced dissipation significantly. For instance, for $w = 2.41$ obtained from
388 calibration, increasing the speed by 100 % results in only 33 % increase in dissipation. In
389 other words, the variation of fuel consumption due to change of speed should generally not
390 be attributed to roughness-induced dissipation.

391 When Gaussian distributions fail to represent the frequency of extreme values and bumps
392 in the road, the found scaling relationships of energy dissipation with road surface param-
393 eters and vehicle dynamic properties remain unchanged. In return, all what changes is the
394 multiplicative factor in the functional form relating energy dissipation and these parameters
395 and properties.

396 **ACKNOWLEDGMENT**

397 This research was carried out by the CSHub@MIT with sponsorship provided by the
398 Portland Cement Association (PCA) and the Ready Mixed Concrete (RMC) Research &
399 Education Foundation. The CSHub@MIT is solely responsible for content.

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494

LIST OF FIGURES

495

1. Quarter-Car model

496

2. Ratio of dimensionless dissipation Π/Π_{GC} versus waviness number w for different β values at (a): $\gamma = 0.1$ (b): $\gamma = 0.2$

497

498

3. Ratio of dimensionless dissipation Π/Π_{GC} versus waviness number w for different γ values at (a): $\beta = 10$ (b): $\beta = 50$

499

500

4. Ratio of dimensionless dissipation Π/Π_{QC} in function of (a): γ and β (b): γ and k_t/k_s

501

5. PDF of road profiles (data from Bruscella et al. (1999)) and the associated normal distribution in (a): linear scale (b): logarithmic scale; the tails of the of empirical distribution are heavier than that of the associated Gaussian distribution

502

503

504

6. PDF of road roughness (data from Bruscella et al. (1999)) and the associated (a): Gaussian distribution (b): Laplace distribution (c): hyperbolic secant distribution (d): logistic distribution

505

506

507

7. Simulated roughness profiles with (a): Gaussian distribution ($c = 3.16 \times 10^{-8}$) (b): Gaussian distribution ($c = 3.16 \times 10^{-6}$) (c): Laplace distribution ($c = 3.16 \times 10^{-8}$) (d): Laplace distribution ($c = 3.16 \times 10^{-6}$)

508

509

510

8. International roughness index in function of unevenness index, c , for $w = 2.5$

511

9. PDF of suspension motion \dot{z} due to roughness with Laplace distribution and unevenness number $c = 3.16 \times 10^{-6}$ along with its associated (a): Gaussian distribution (b): Laplace distribution (c): hyperbolic secant distribution (d): logistic distribution

512

513

514

10. PDF of suspension motion \dot{z} due to roughness with Laplace distribution and unevenness number $c = 3.16 \times 10^{-8}$ along with its associated (a): Gaussian distribution (b): Laplace distribution (c): hyperbolic secant distribution (d): logistic distribution

515

516

517

11. Change in roughness-induced excess fuel consumption in function of IRI at $V = 70$

518 and 100 km/h for (a): Medium car (b): SUV (c): Van (d):Light truck (e): Articulated
519 truck

520 **List of Tables**

521 1 Properties of the golden-car (data from Sayers (1995)) 29

522 2 Values of α in $E[\text{IRI}] = \alpha\sqrt{c}$ and k_d in Eq. (24) for various distributions . . 30

523 3 Vehicle dynamic properties per axel 31

TABLE 1: Properties of the golden-car (data from Sayers (1995))

| Property | Value | Units |
|-----------|-------|-------------------|
| k_t/m_s | 653 | $[\text{s}^{-2}]$ |
| k_s/m_s | 63.3 | $[\text{s}^{-2}]$ |
| C_s/m_s | 6.0 | $[\text{s}^{-1}]$ |
| m_u/m_s | 0.15 | [1] |

TABLE 2: Values of α in $E[\text{IRI}] = \alpha\sqrt{c}$ and k_d in Eq. (24) for various distributions

| Probability distribution | α | k_d |
|--------------------------|----------|-----------------------|
| Gaussian | 1.9154 | $\sqrt{\pi}$ |
| Logistic | 1.8348 | $\pi/\sqrt{6}\ln 2$ |
| Hyperbolic Secant | 1.7823 | $\sqrt{2}\pi^2/7.328$ |
| Laplace | 1.6975 | 2 |

TABLE 3: Vehicle dynamic properties per axel

| Vehicle class | Medium car | SUV | Van | Light truck | Articulated truck |
|---|--------------------|-------------------|-------------------|-------------------|-------------------|
| Total mass, m_t (tons) | 1.46 ¹ | 2.5 ¹ | 2.54 ¹ | 6.5 ¹ | 34.9 ¹ |
| Unsprung mass, m_u (kg) | 80 ² | 125 ³ | 134 ⁷ | 395 ⁵ | 544 ⁶ |
| Suspension stiffness k_s (kN/m) | 29.44 ² | 189 ³ | 48 ⁴ | 337 ⁵ | 700 ⁶ |
| Fuel efficiency coefficient ξ_b (mL/kW/s) | .096 ¹ | .072 ¹ | .072 ¹ | .062 ¹ | .059 ¹ |
| β (Gaussian) | 46.98 | 28.03 | 31.00 | 14.90 | 13.30 |
| β (Logistic) | 44.46 | 26.51 | 29.32 | 13.95 | 12.54 |
| β (Hyperbolic secant) | 42.84 | 25.53 | 28.24 | 13.42 | 12.04 |
| β (Laplace) | 40.24 | 23.95 | 26.51 | 12.57 | 11.26 |

- 1 - Chatti and Zaabar (2012)
- 2 - Dixon (1996)
- 3 - CarSim template
- 4 - Dastun1200 (2013)
- 5 - GMC Specification manual
- 6 - Fancher (1986)
- 7 - Winkler (1983)

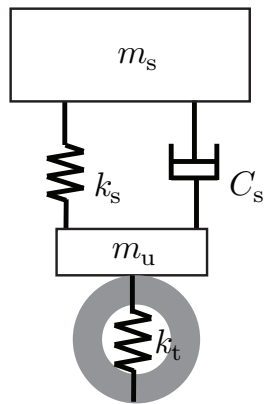
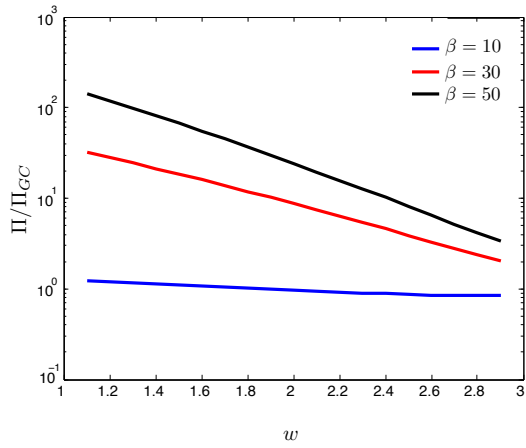
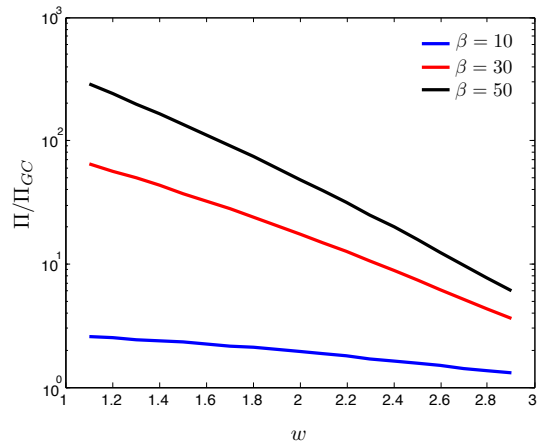


FIG. 1: Quarter-Car model adapted from Sayers (1995)

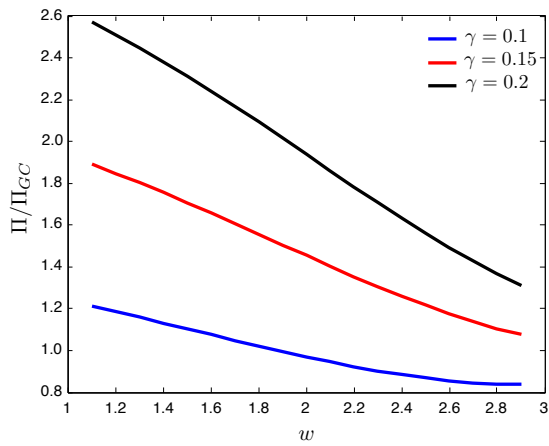


(a) $\gamma = 0.1$

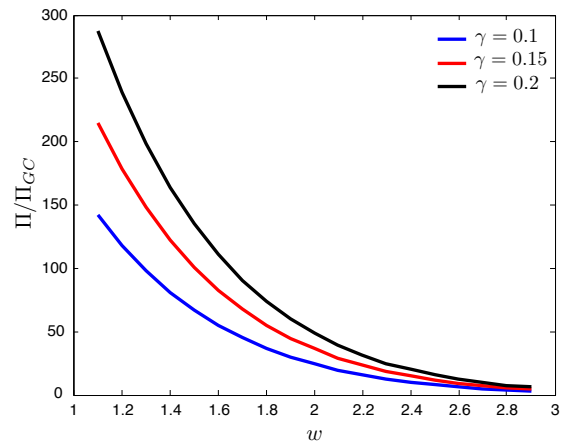


(b) $\gamma = 0.2$

FIG. 2: Ratio of dimensionless dissipation Π/Π_{GC} versus waviness number w for different β values at (a): $\gamma = 0.1$ (b): $\gamma = 0.2$



(a) $\beta = 10$



(b) $\beta = 50$

FIG. 3: Ratio of dimensionless dissipation Π/Π_{GC} versus waviness number w for different γ values at (a): $\beta = 10$ (b): $\beta = 50$

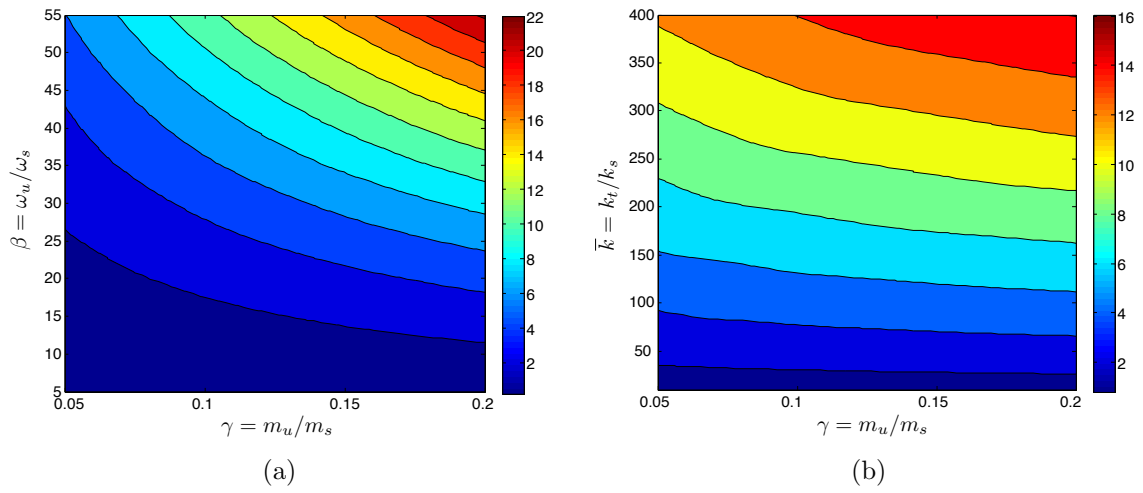


FIG. 4: Ratio of dimensionless dissipation Π/Π_{QC} in function of (a): γ and β (b): γ and k_t/k_s

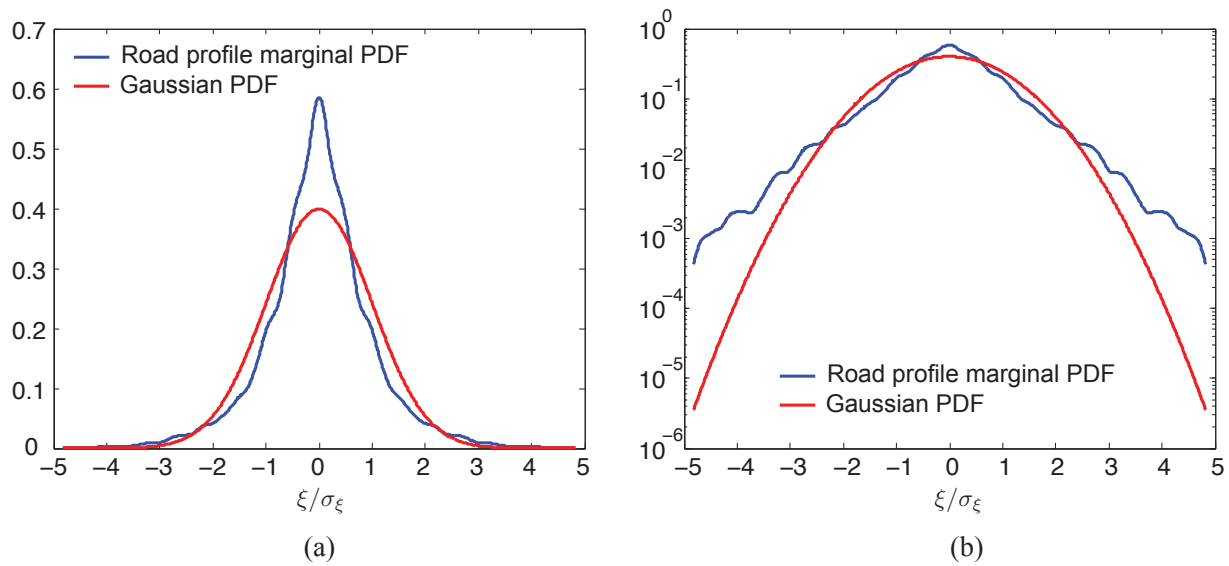
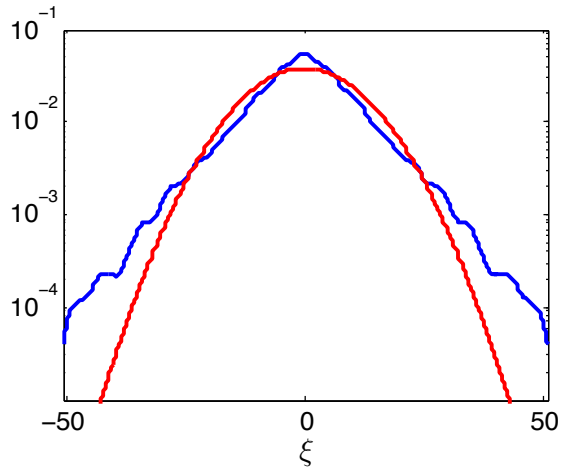
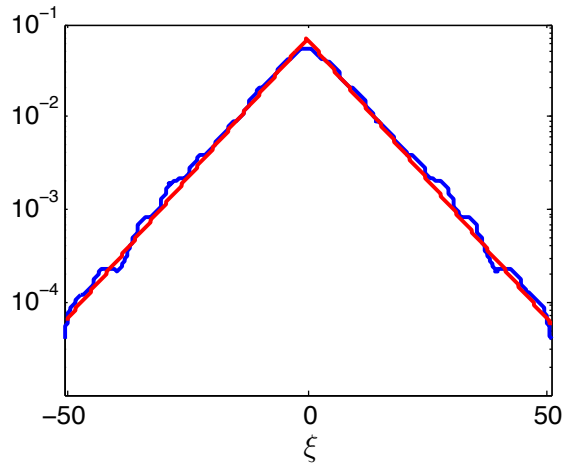


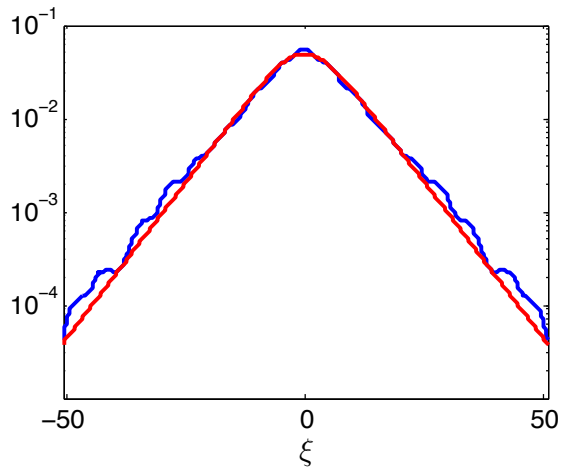
FIG. 5: PDF of road profiles adapted from Bruscella et al. (1999) and the associated normal distribution in (a): linear scale (b): logarithmic scale; the tails of the of empirical distribution are heavier than that of the associated Gaussian distribution



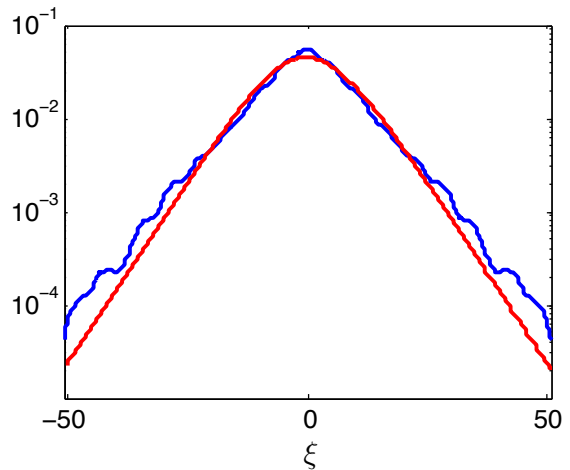
(a) Normal distribution



(b) Laplace distribution



(c) Hyperbolic secant distribution



(d) Logistic distribution

FIG. 6: PDF of road roughness adapted from Bruscella et al. (1999) and the associated (a): Gaussian distribution (b): Laplace distribution (c): hyperbolic secant distribution (d): logistic distribution

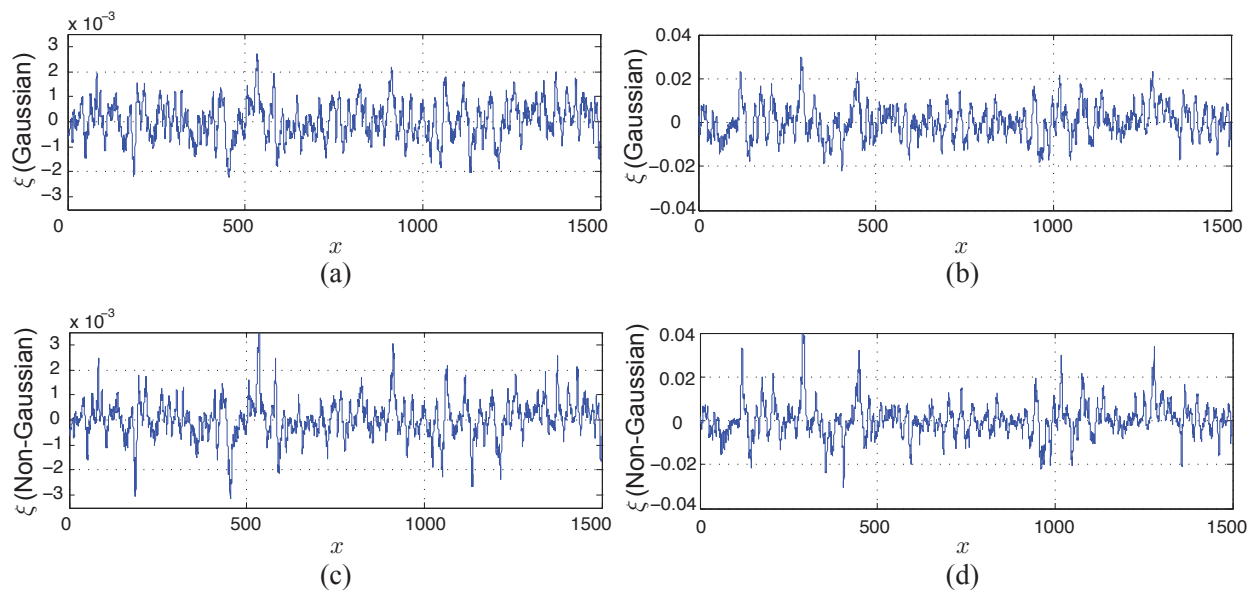


FIG. 7: Simulated roughness profiles with (a): Gaussian distribution ($c = 3.16 \times 10^{-8}$) (b): Gaussian distribution ($c = 3.16 \times 10^{-6}$) (c): Laplace distribution ($c = 3.16 \times 10^{-8}$) (d): Laplace distribution ($c = 3.16 \times 10^{-6}$)

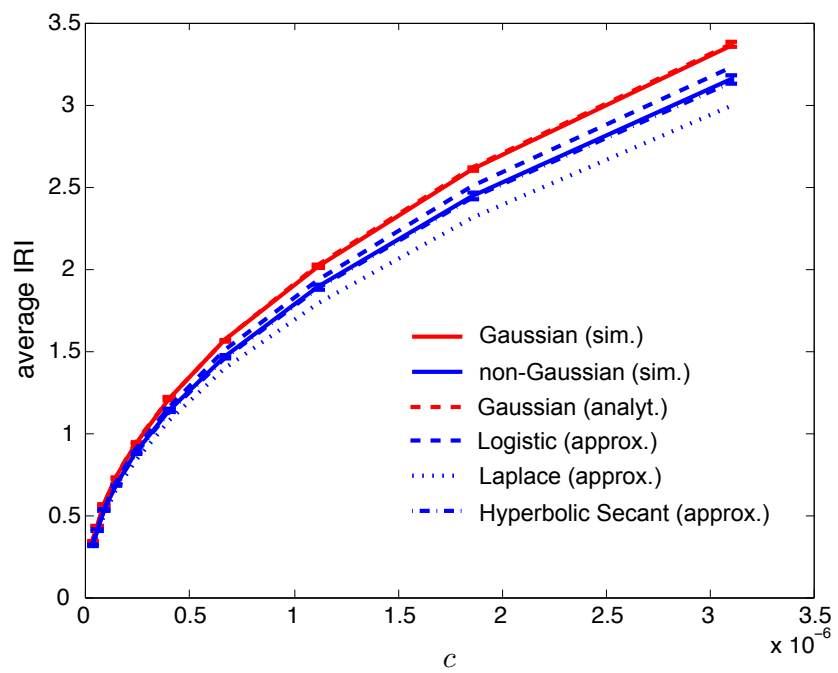
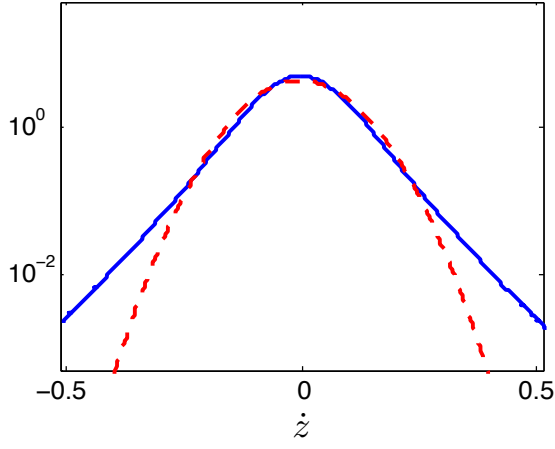
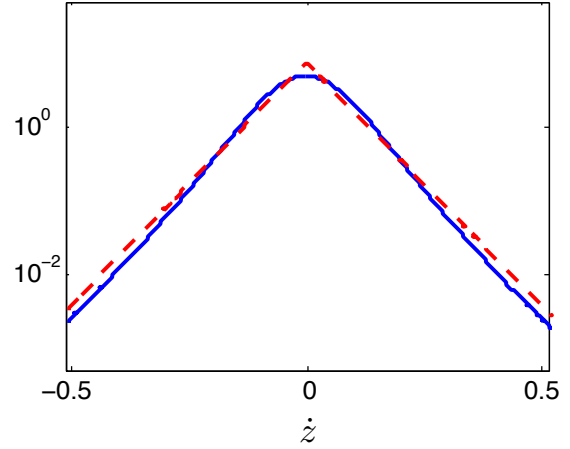


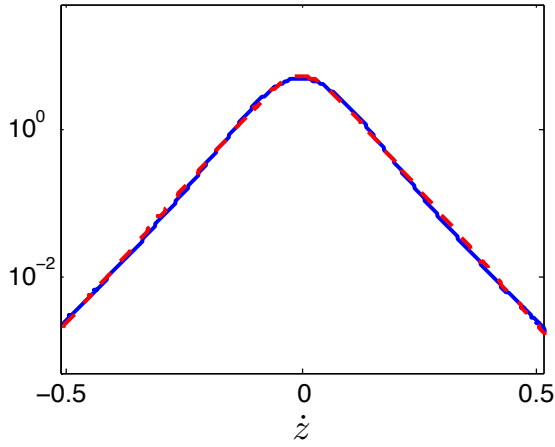
FIG. 8: International roughness index in function of unevenness index, c , for $w = 2.5$



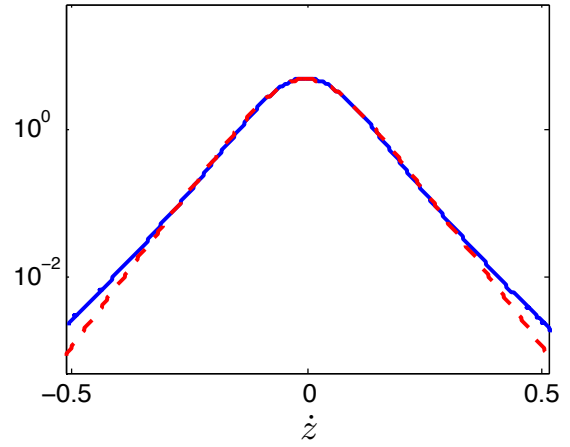
(a) Normal distribution



(b) Laplace distribution

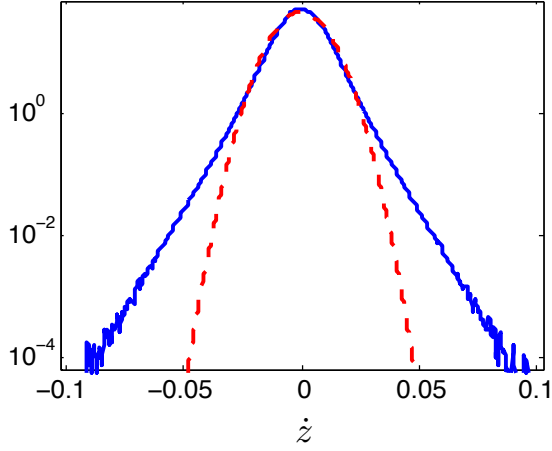


(c) Hyperbolic secant distribution

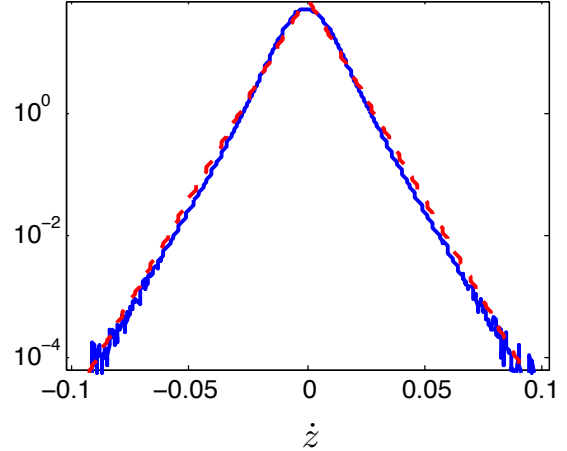


(d) Logistic distribution

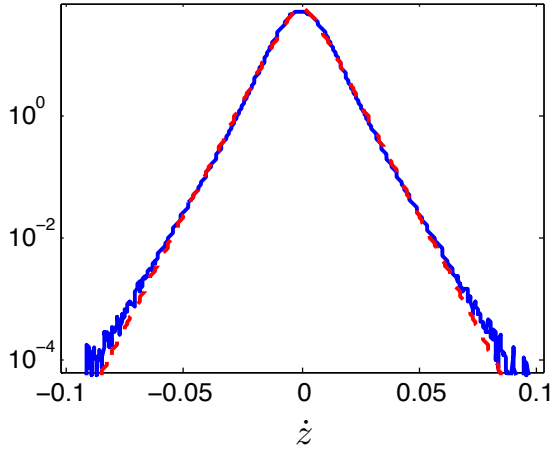
FIG. 9: PDF of suspension motion \dot{z} due to roughness with Laplace distribution and unevenness number $c = 3.16 \times 10^{-6}$ along with its associated (a): Gaussian distribution (b): Laplace distribution (c): hyperbolic secant distribution (d): logistic distribution



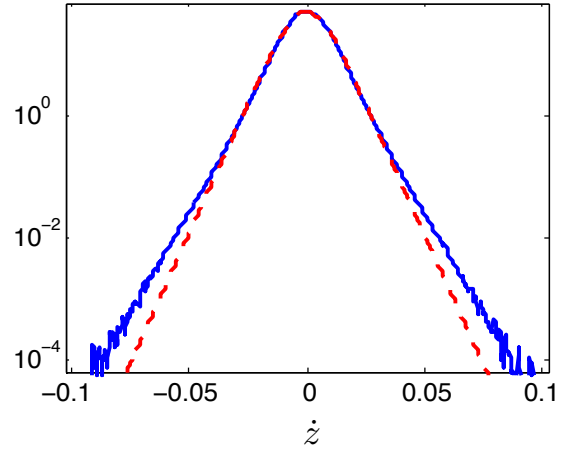
(a) Normal distribution



(b) Laplace distribution

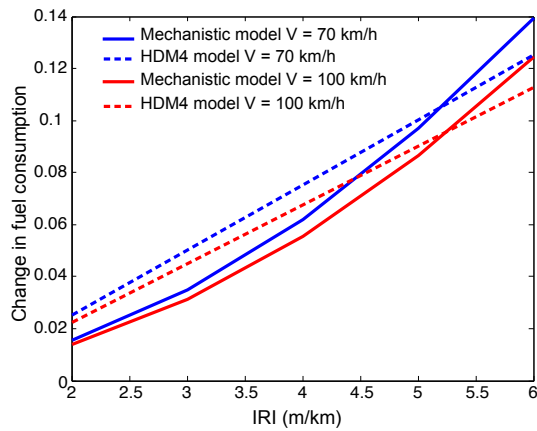


(c) Hyperbolic secant distribution

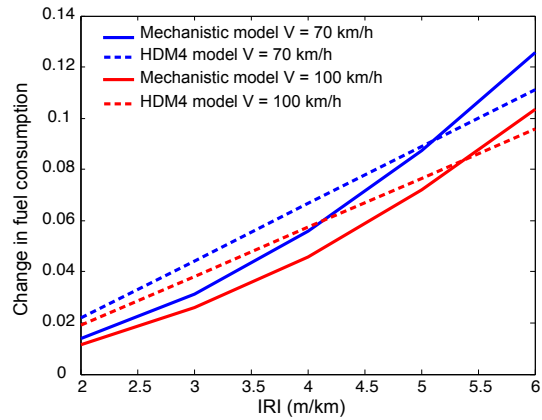


(d) Logistic distribution

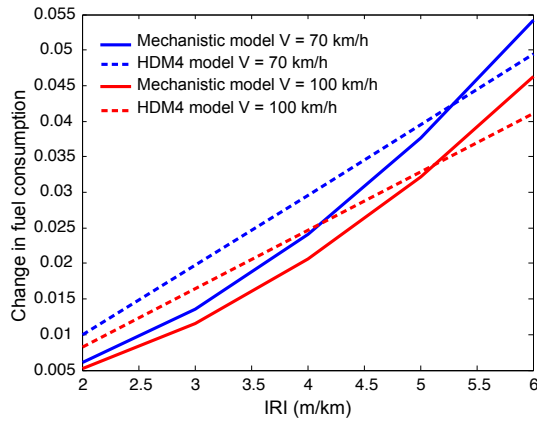
FIG. 10: PDF of suspension motion \dot{z} due to roughness with Laplace distribution and unevenness number $c = 3.16 \times 10^{-8}$ along with its associated (a): Gaussian distribution (b): Laplace distribution (c): hyperbolic secant distribution (d): logistic distribution



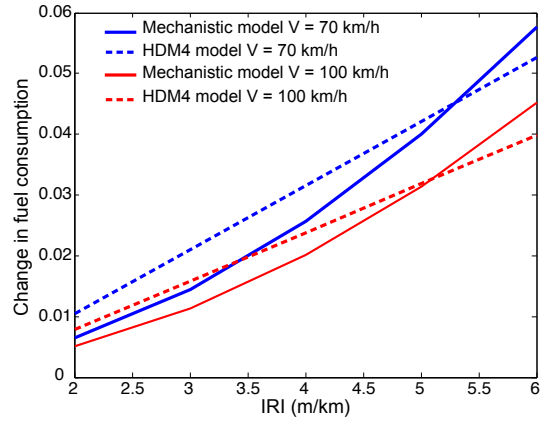
(a) Medium Car



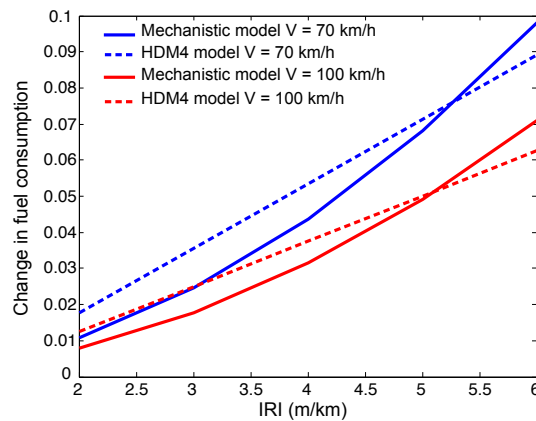
(b) SUV



(c) Van



(d) Light Truck



(e) Articulated Truck

FIG. 11: Change in roughness-induced excess fuel consumption in function of IRI at $V = 70$ and 100 km/h for (a): Medium car (b): SUV (c): Van (d):Light truck (e): Articulated truck