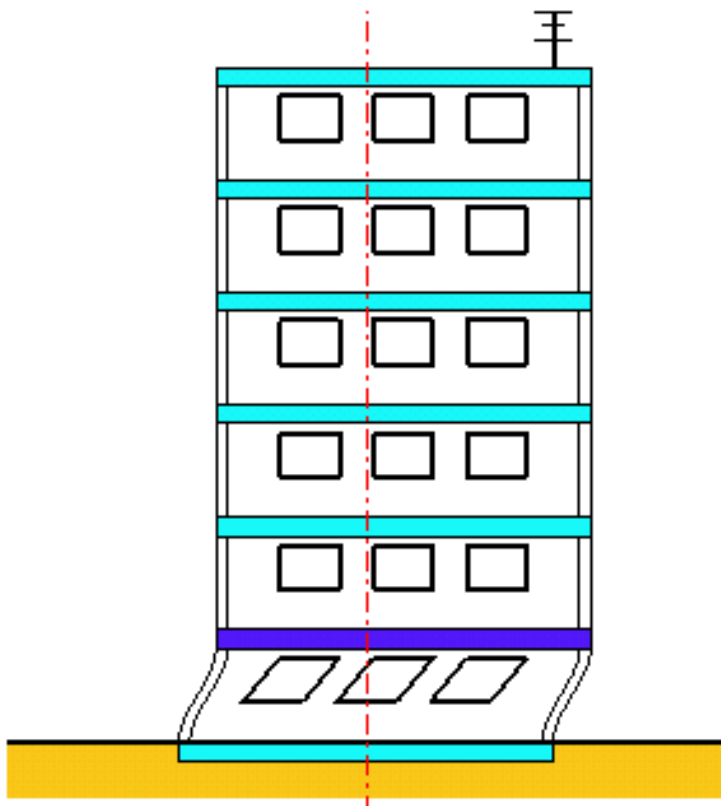
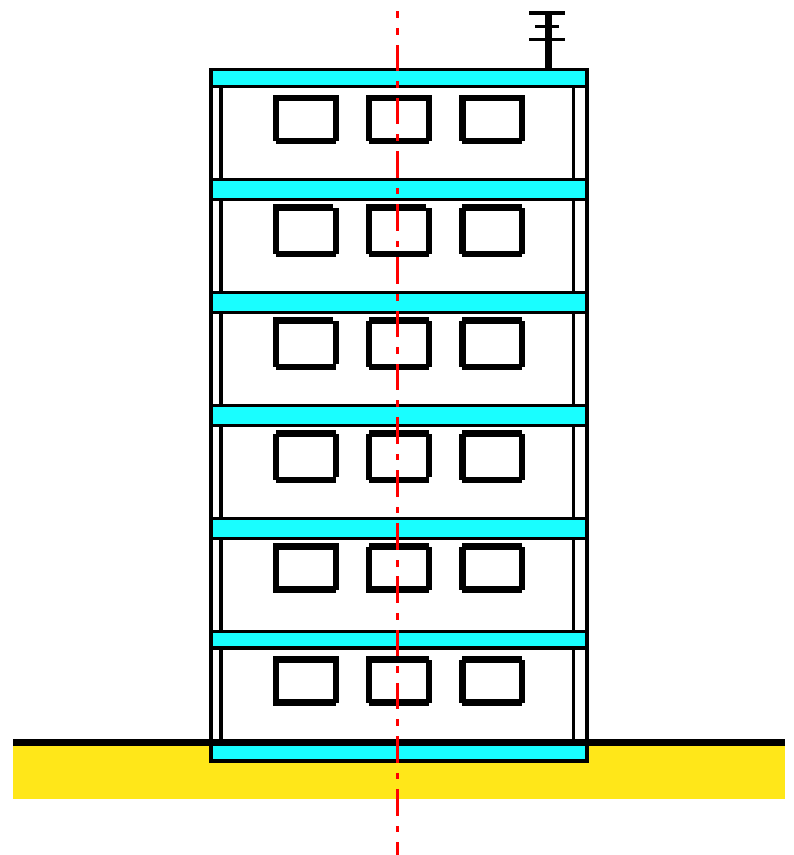


Loading + Failure

Wind load hitting Skyscraper

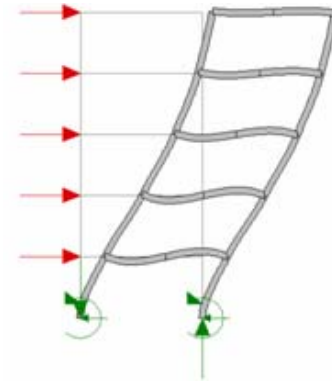


Earthquake hitting Skyscraper



Lateral Load Systems

1. Frame → rigid joints



2. Bracing → diagonal elements



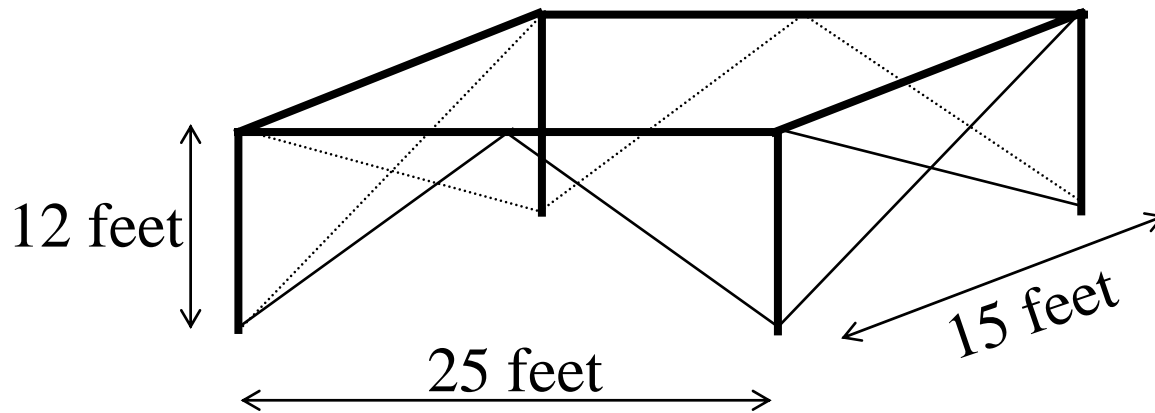
3. Shear wall → diaphragm



Good Practice in Wind Design

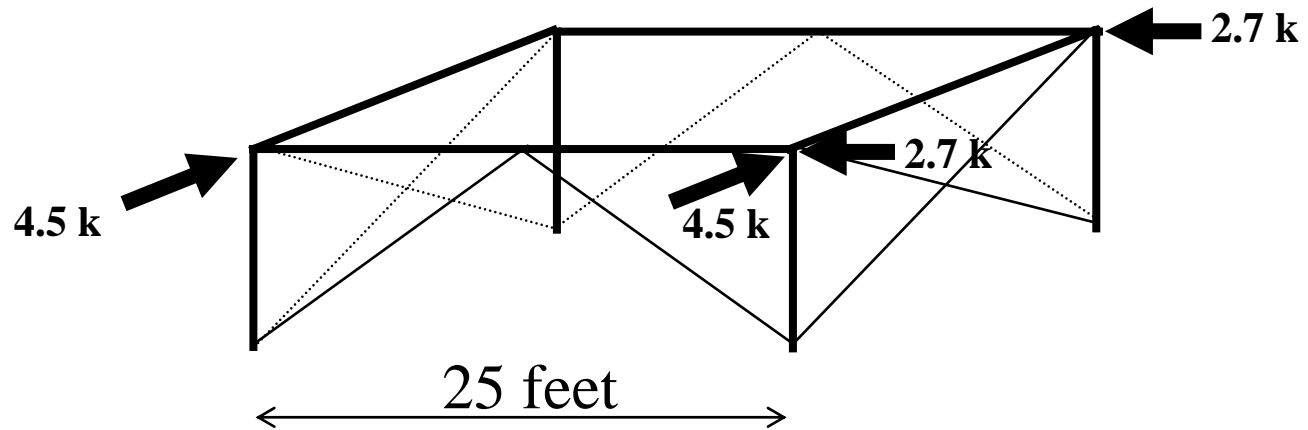
- **Provide greater resistance along shorter dimension**
- **Careful attention to detail**
 - **Continuity of structure**
 - **Provide ties for each element**
- **Greater weight can provide greater wind resistance**
- **Protect inhabitants from flying projectiles**

Wind Bracing Example



- **Wind loading of 30 pounds per square foot applied on each side**

Wind Bracing Example



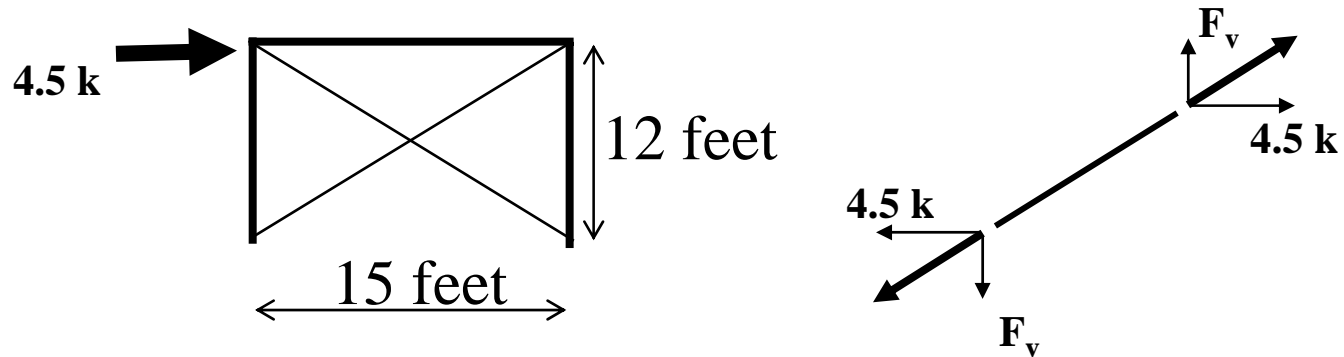
$$(25 \text{ ft})(12 \text{ ft})(30 \text{ psf}) = 9,000 \text{ lbs}$$

Applied at top of wall, 4,500 lbs/frame (4.5 k)

$$(15 \text{ ft})(12 \text{ ft})(30 \text{ psf}) = 5,400 \text{ lbs}$$

Applied at top of wall, 2,700 lbs/frame (2.7 k)

Wind Bracing Example

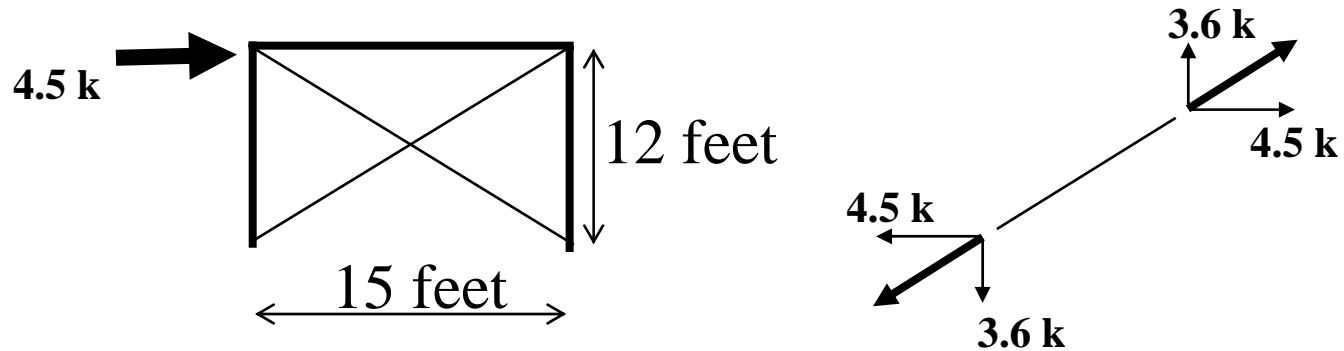


If the cable carries 100% of wind load, then the horizontal component of force in the cable must be 4.5 k

$$F_v / (4.5 \text{ k}) = (12 \text{ ft}) / (15 \text{ ft})$$

$$F_v = 3.6 \text{ k}$$

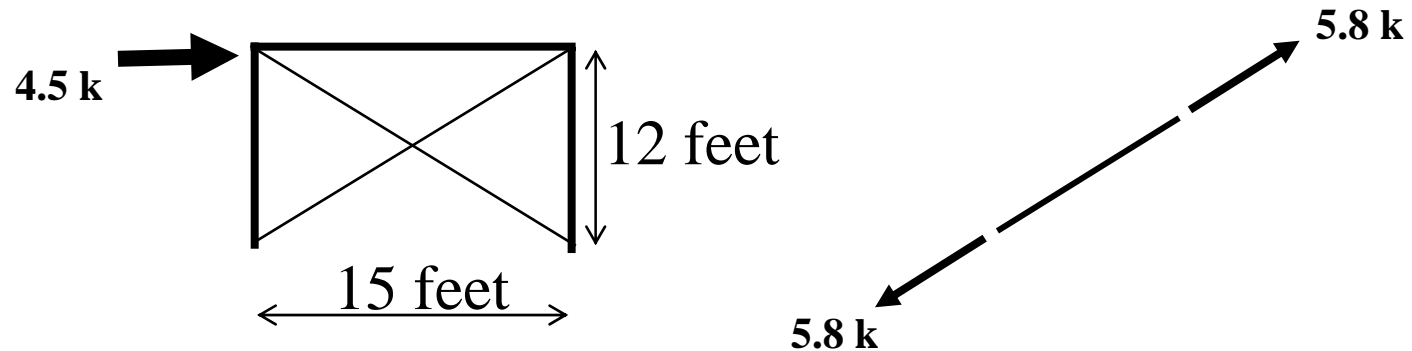
Wind Bracing Example



**Internal force in the cable is found from
Pythagorean Theorem**

$$F_{\text{cable}} = \sqrt{(4.5 \text{ k})^2 + (3.6 \text{ k})^2} = 5.8 \text{ k}$$

Required Area of Steel Cable



Maximum allowable stress in cable is 15 ksi (given)

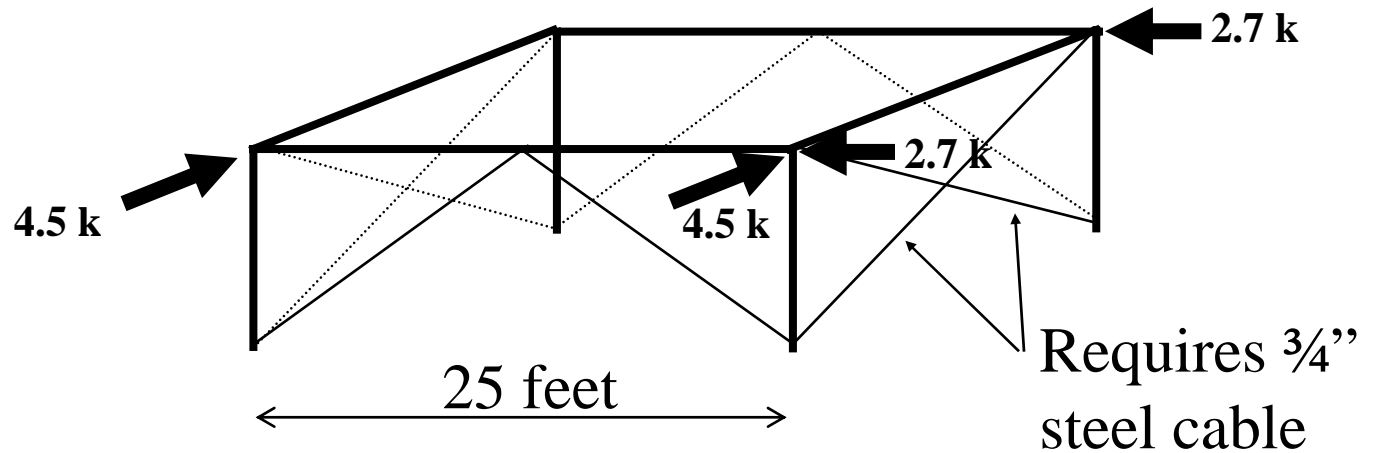
Stress = Force/Area

$$A_{\text{req'd}} = \text{Force}/\text{Stress} = 5.8 \text{ k}/15 \text{ ksi} = 0.38 \text{ in}^2$$

Cable diameter, $d = 0.7$ inches ($A = \pi(0.7)^2/4 = 0.38 \text{ in}^2$)

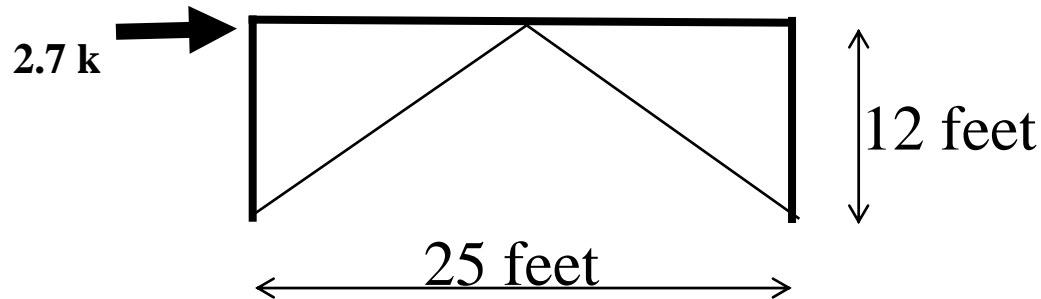
Specify $3/4$ " steel cable ($A = 0.44 \text{ in}^2$)

Wind Bracing Example



What about bracing in other direction?

Wind Bracing Example

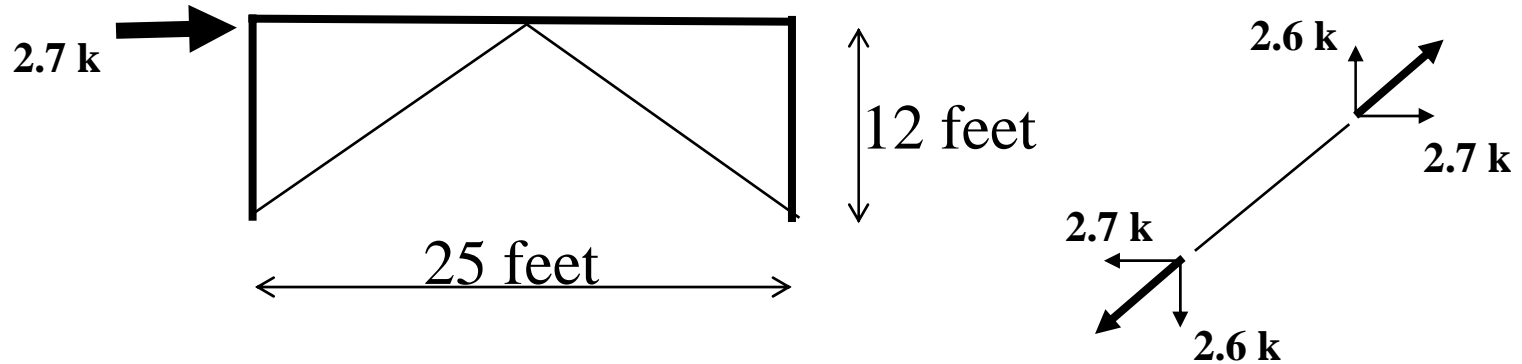


Assume bracing carries 100% of wind load, then the horizontal component of force in the cable must be 2.7 k

$$F_v / (2.7 \text{ k}) = (12 \text{ ft}) / (12.5 \text{ ft})$$

$$F_v = 2.6 \text{ k}$$

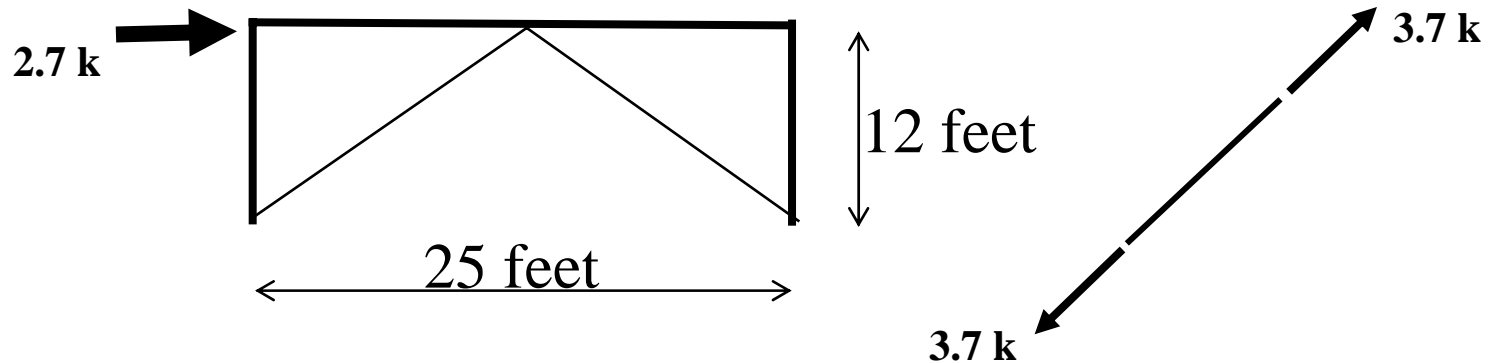
Wind Bracing Example



**Internal force in the cable is found from
Pythagorean Theorem**

$$F_{\text{cable}} = \sqrt{(2.7 \text{ k})^2 + (2.6 \text{ k})^2} = 3.7 \text{ k}$$

Required Area of Steel Cable



Maximum allowable stress in cable is 15 ksi (given)

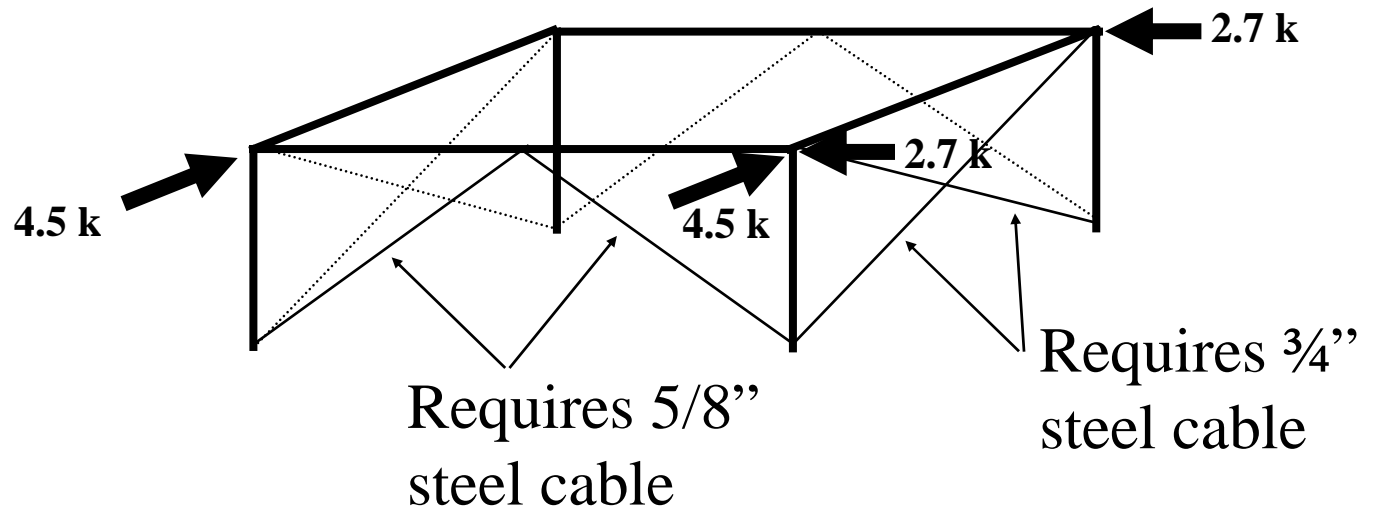
Stress = Force/Area

$$A_{\text{req'd}} = \text{Force}/\text{Stress} = 3.7 \text{ k}/15 \text{ ksi} = 0.25 \text{ in}^2$$

Cable diameter, $d = 0.56$ inches ($A = \pi(0.56)^2/4 = 0.25 \text{ in}^2$)

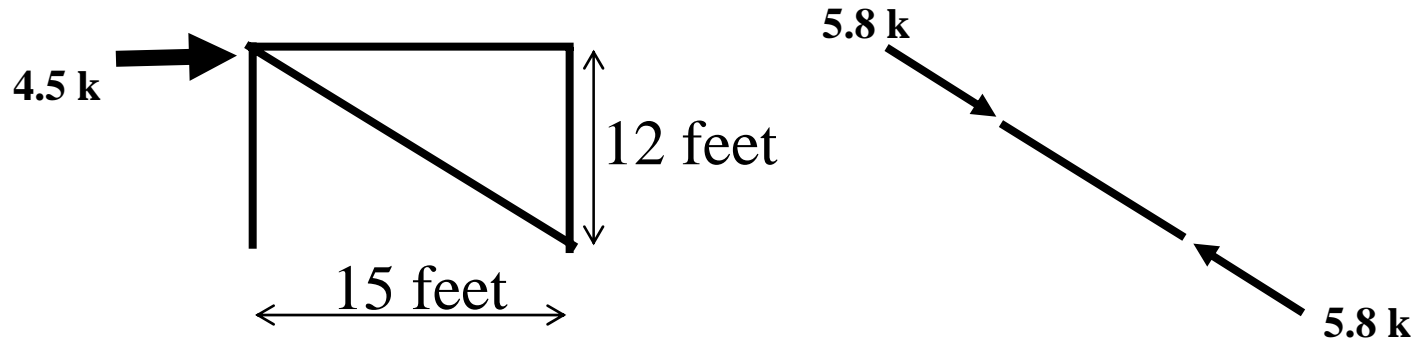
Specify 5/8" steel cable ($A = 0.31 \text{ in}^2$)

Wind Bracing Example



Design of wind bracing for cables

Compression Element as Wind Bracing

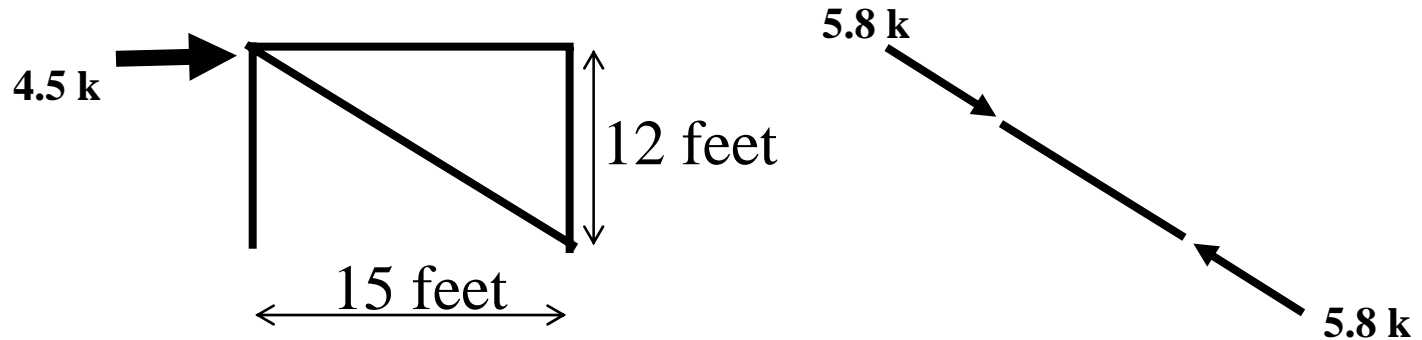


Now one element must carry 5.8 k in both tension and compression

For tension, must have an area of 0.38 in² (3/4 in diameter bar)

So cross-sectional area must be greater than 0.38 in²

Compression Element as Wind Bracing



**Compression element requires about 6 times the material
(to resist buckling)**

Design Considerations

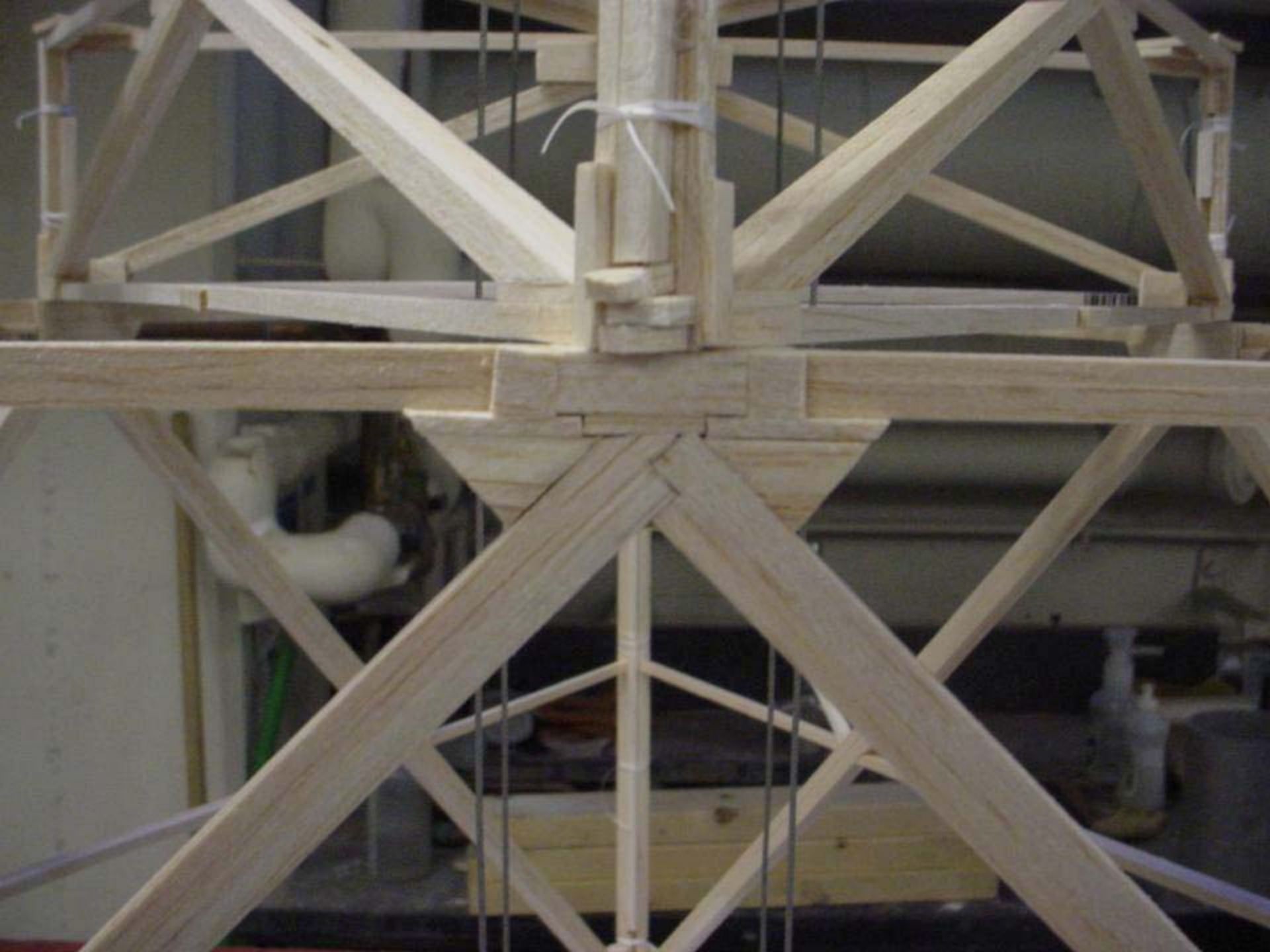
- **Tension braces use less material than compression braces for lateral loads**
- **Compression members must resist buckling which requires extra material**
- **Must consider buckling about either axis, and should brace the weak axis**

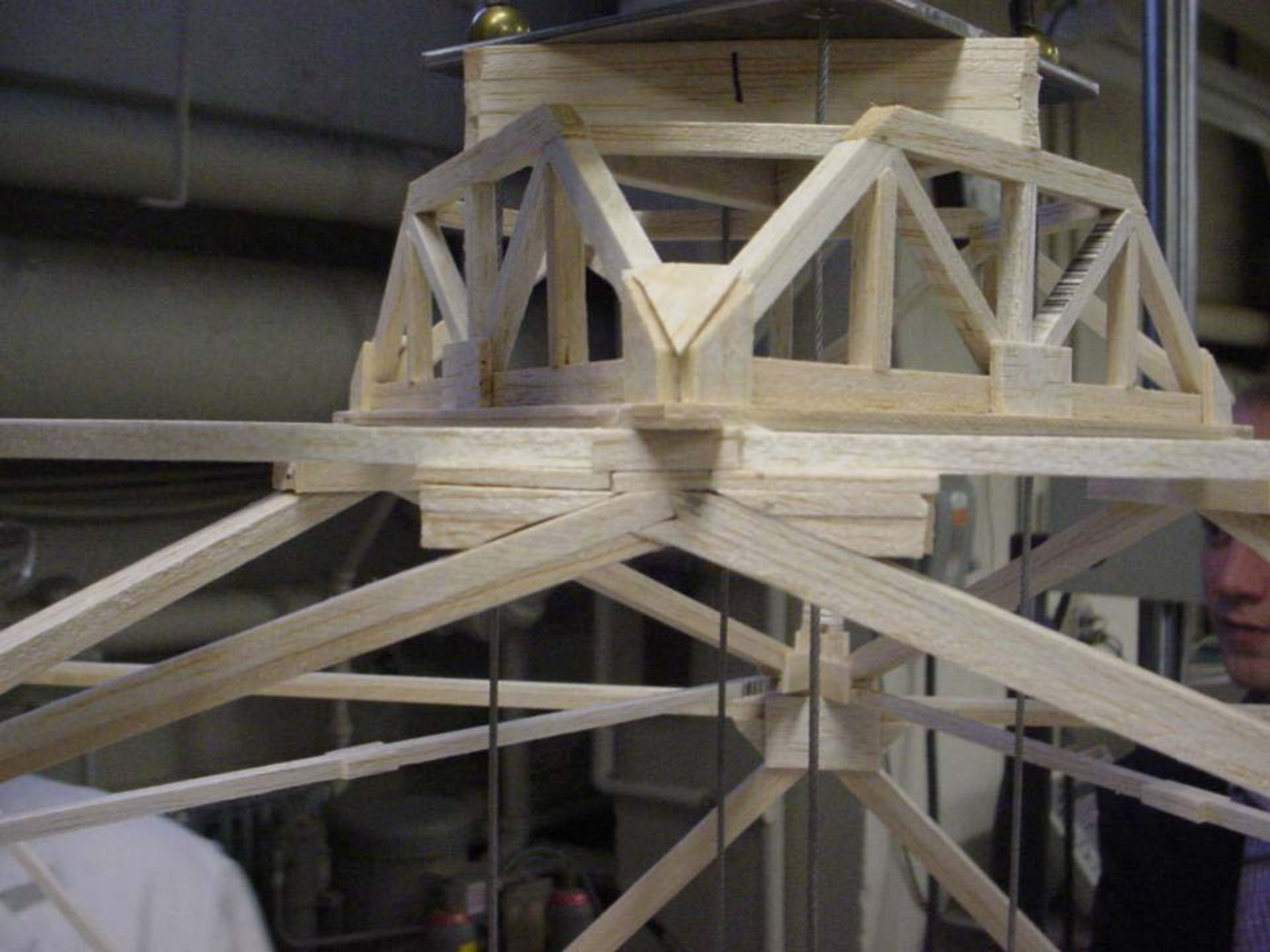
Design Considerations

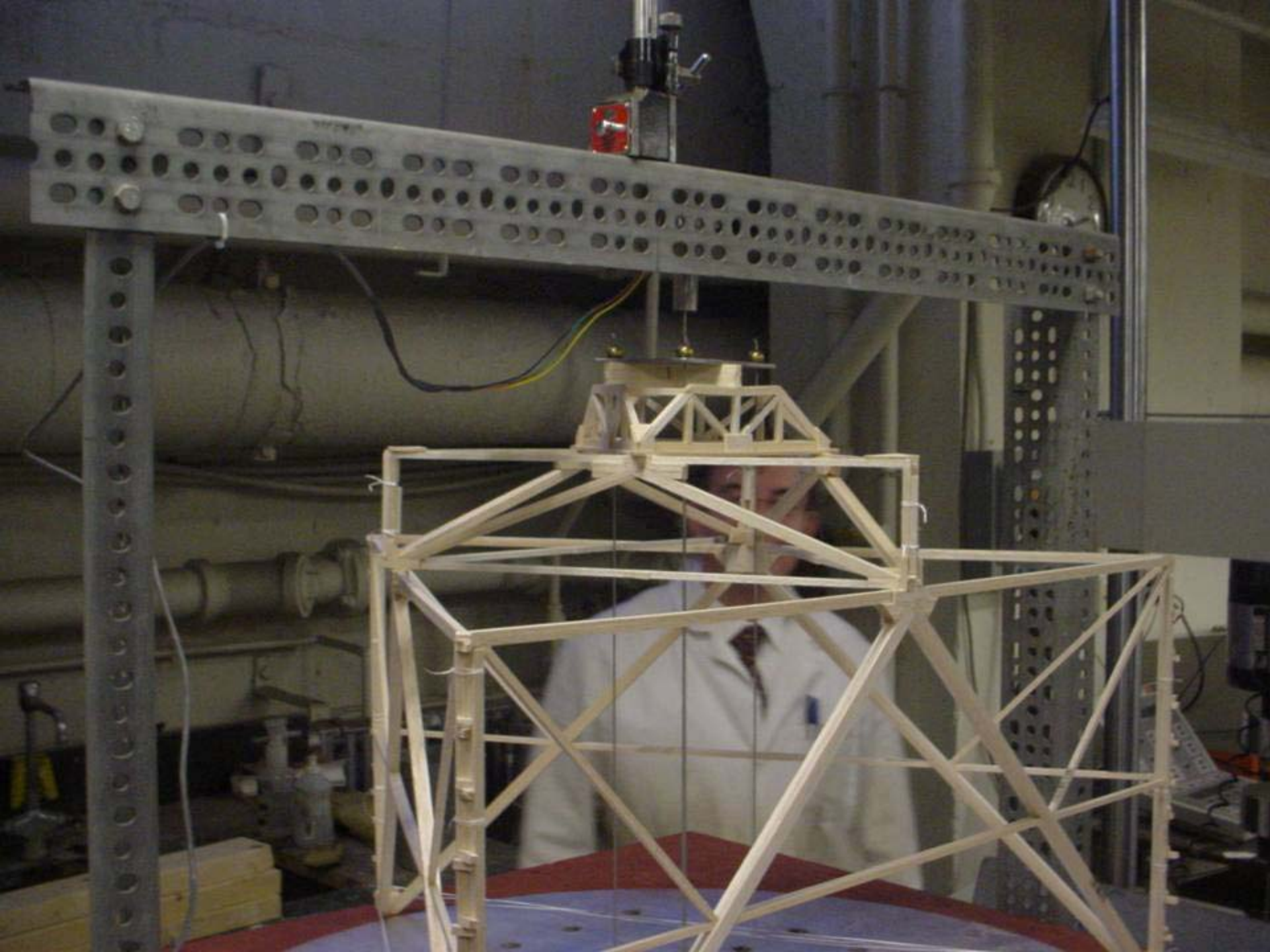
- **Want to have ductility in our structures so that sudden collapse does not occur**
- **Failure by buckling may be sudden and unexpected → not a ductile failure**
- **Statically determinate structures are easier to design because you can know the internal forces exactly**

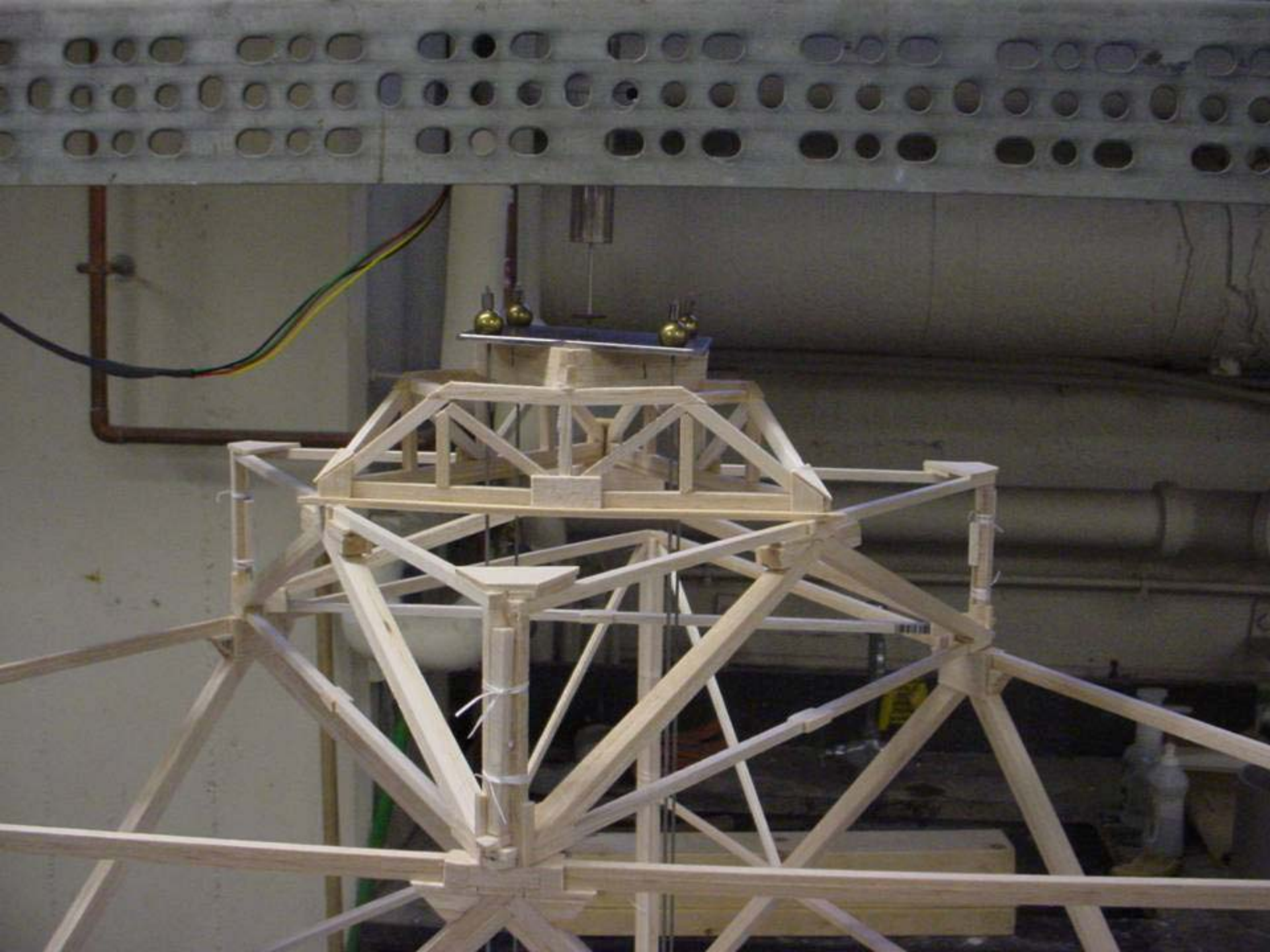
Balsa Dancing

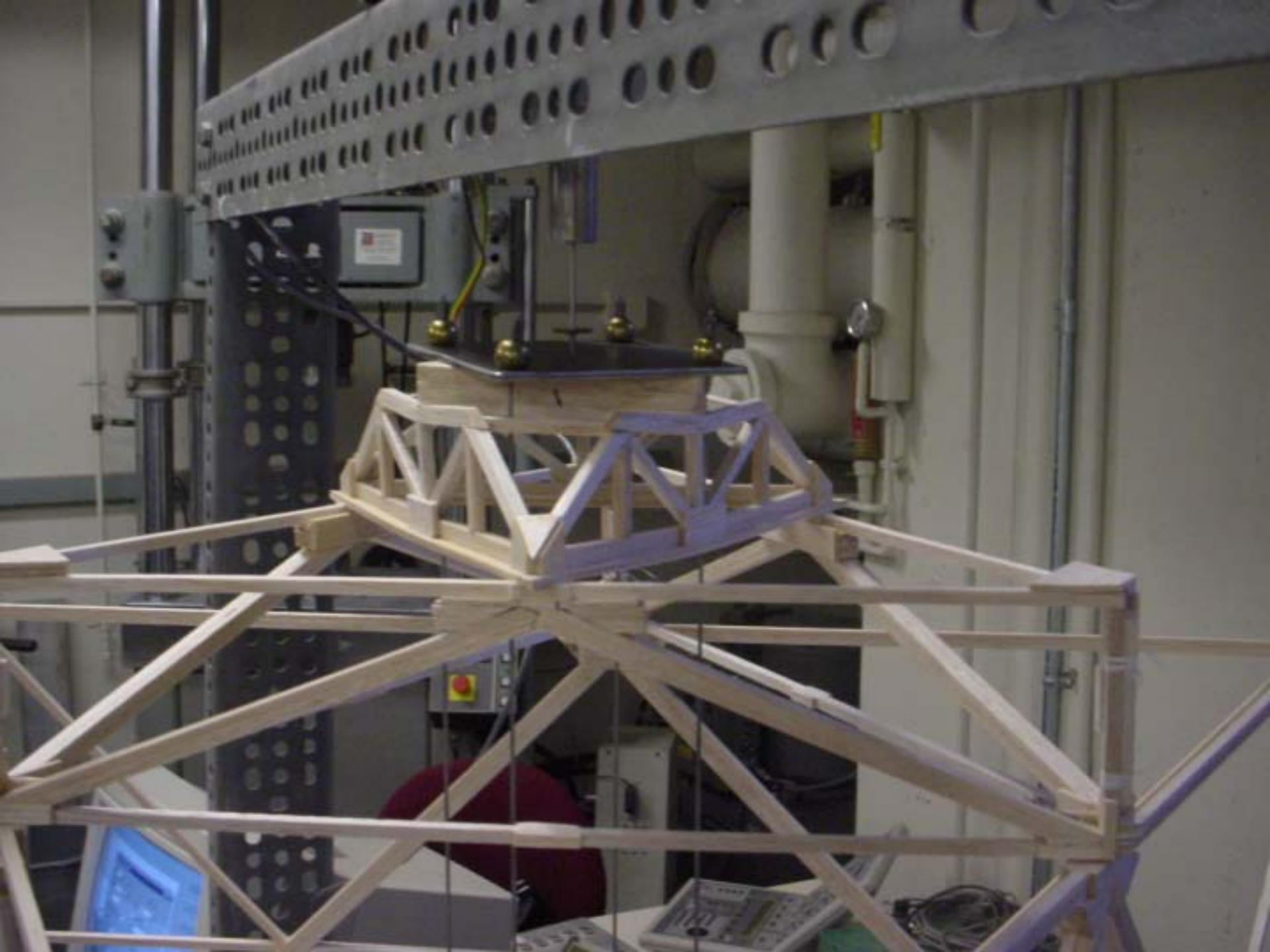


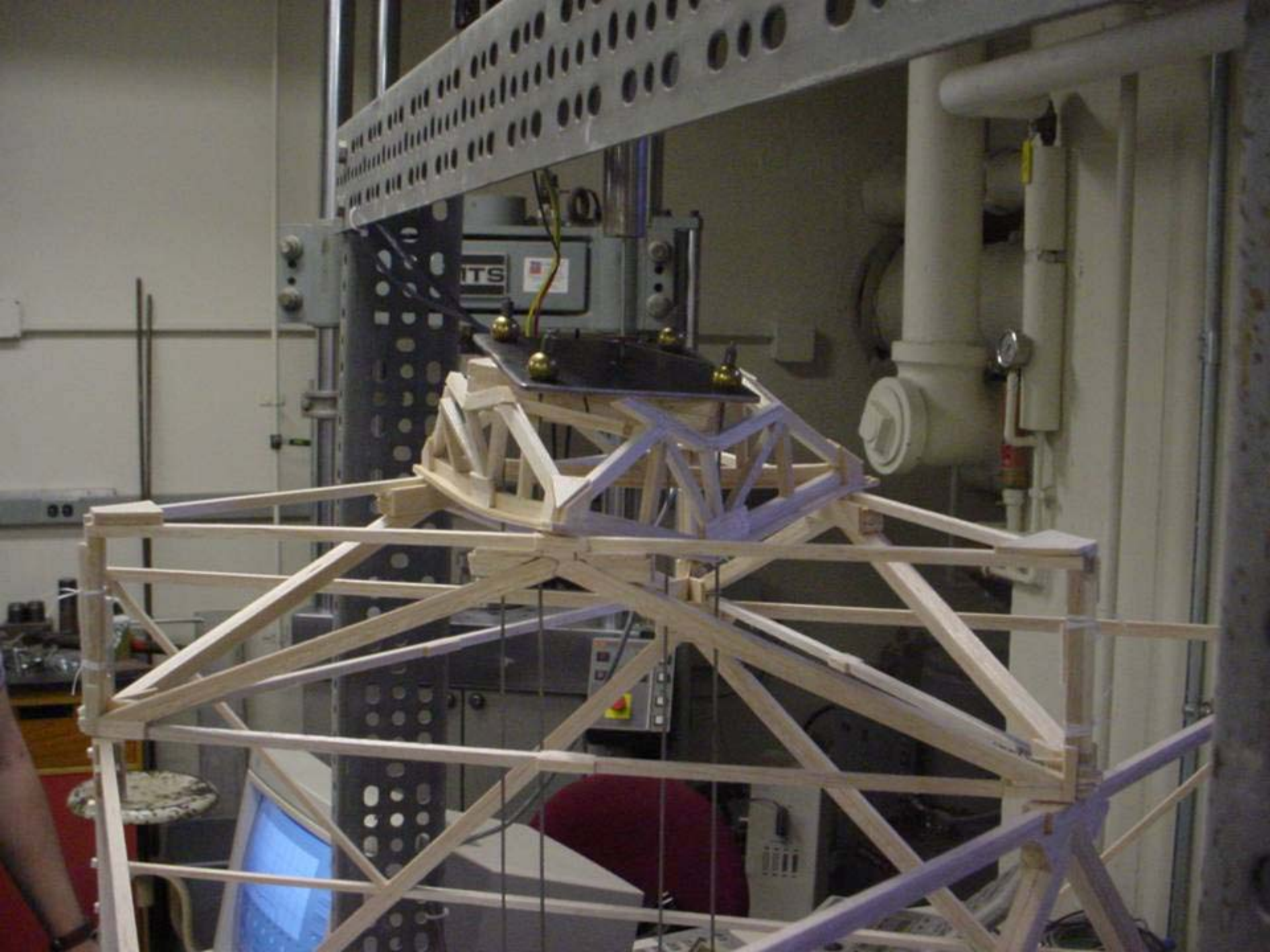


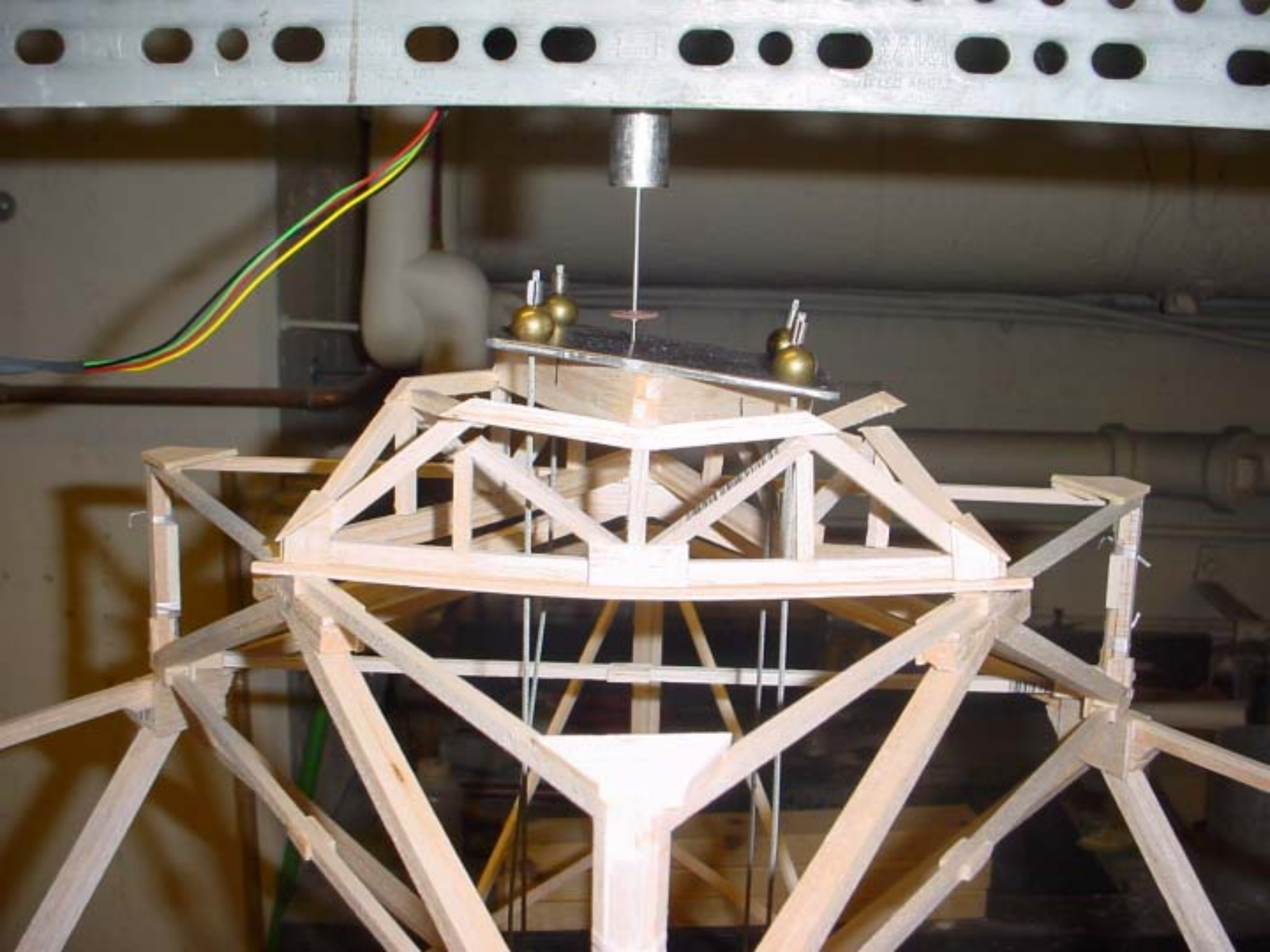


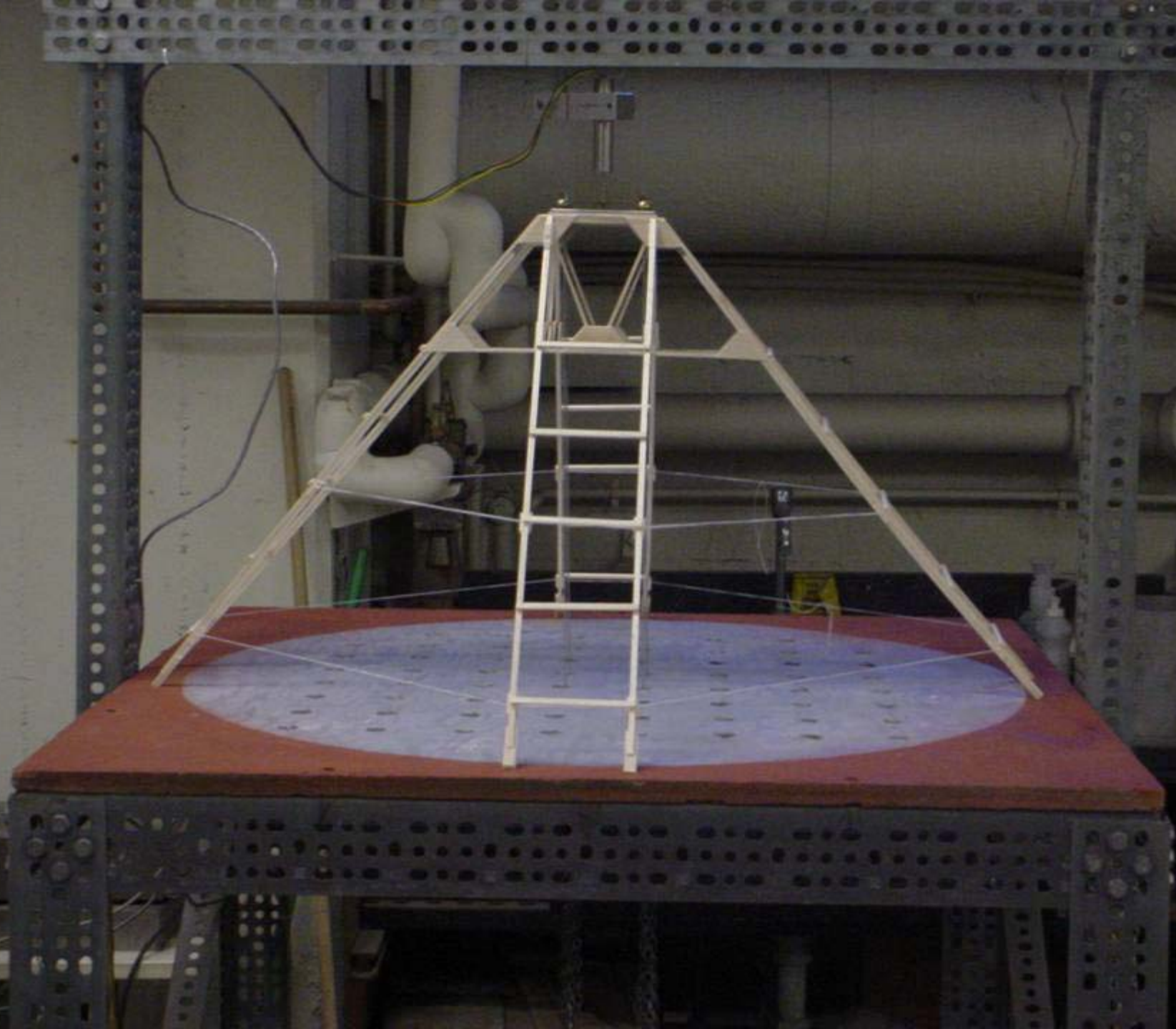


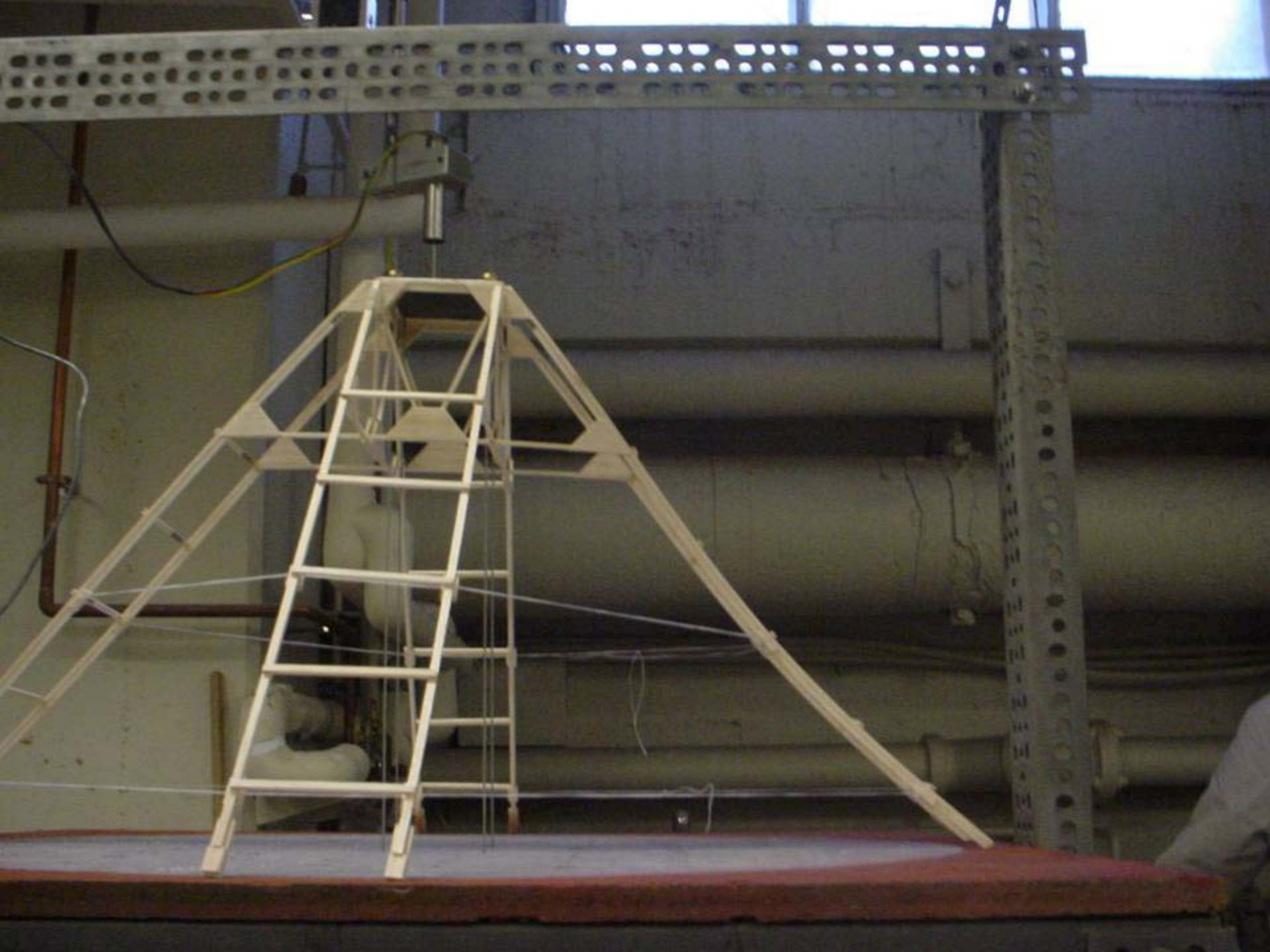


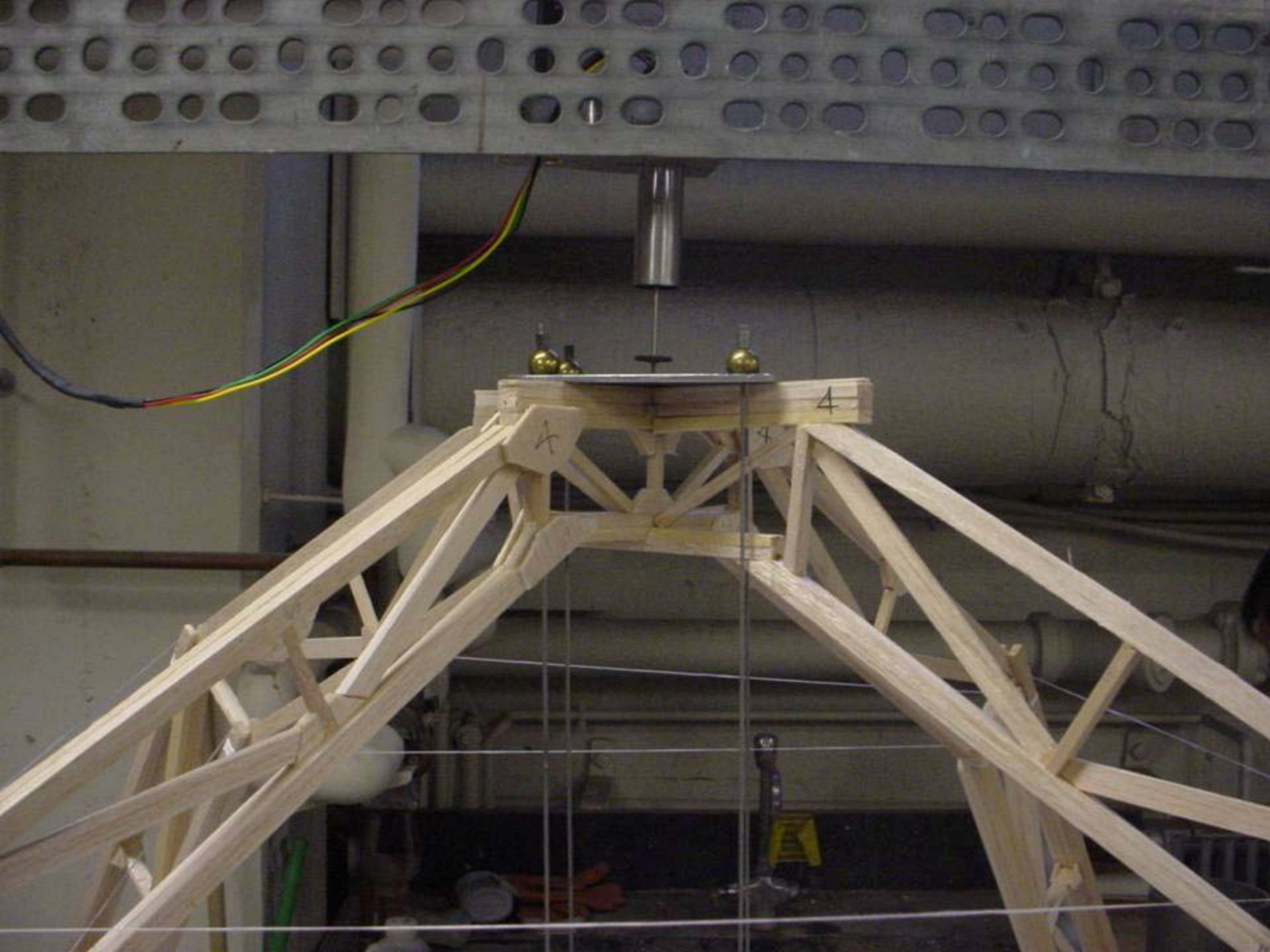


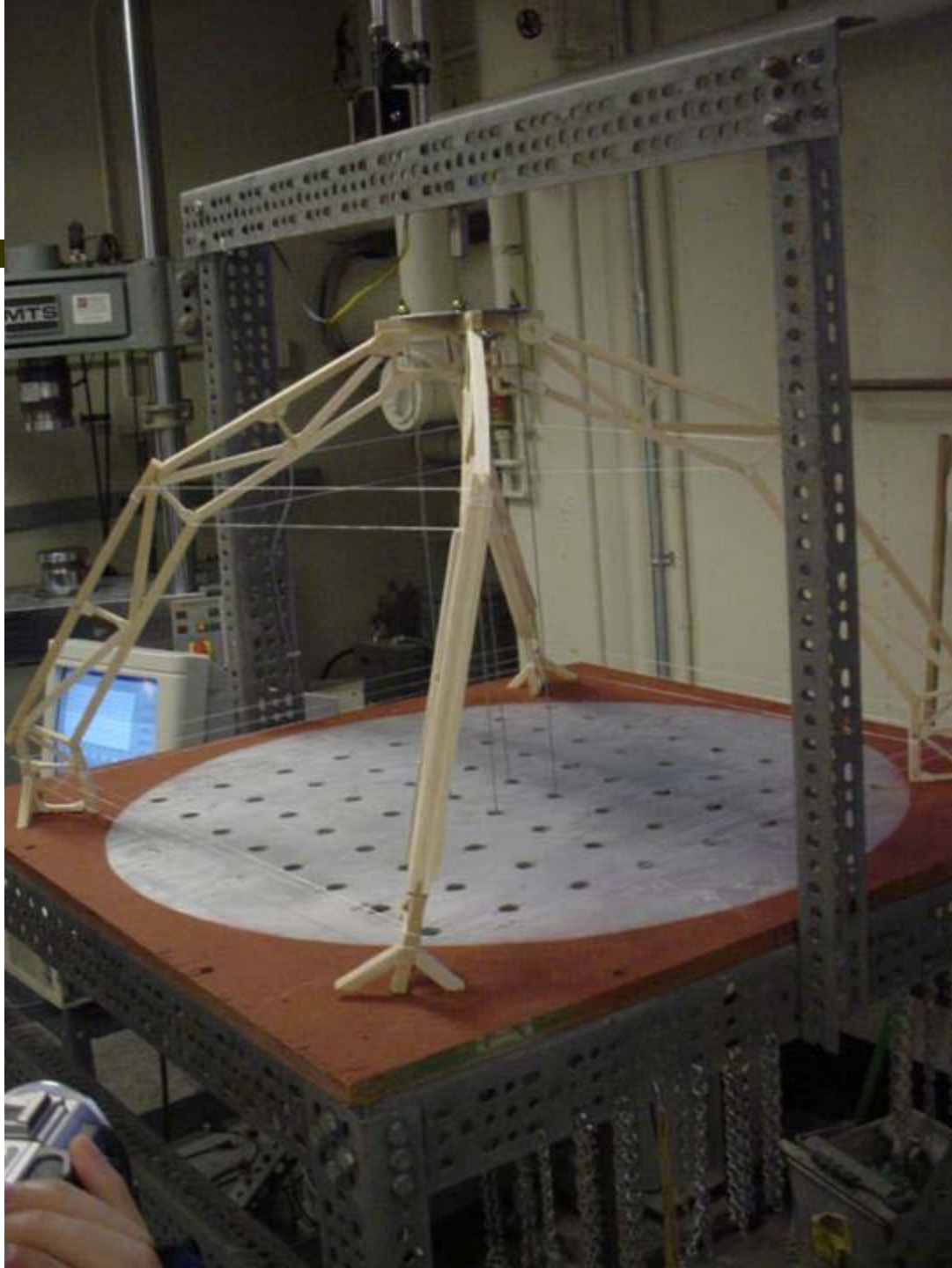


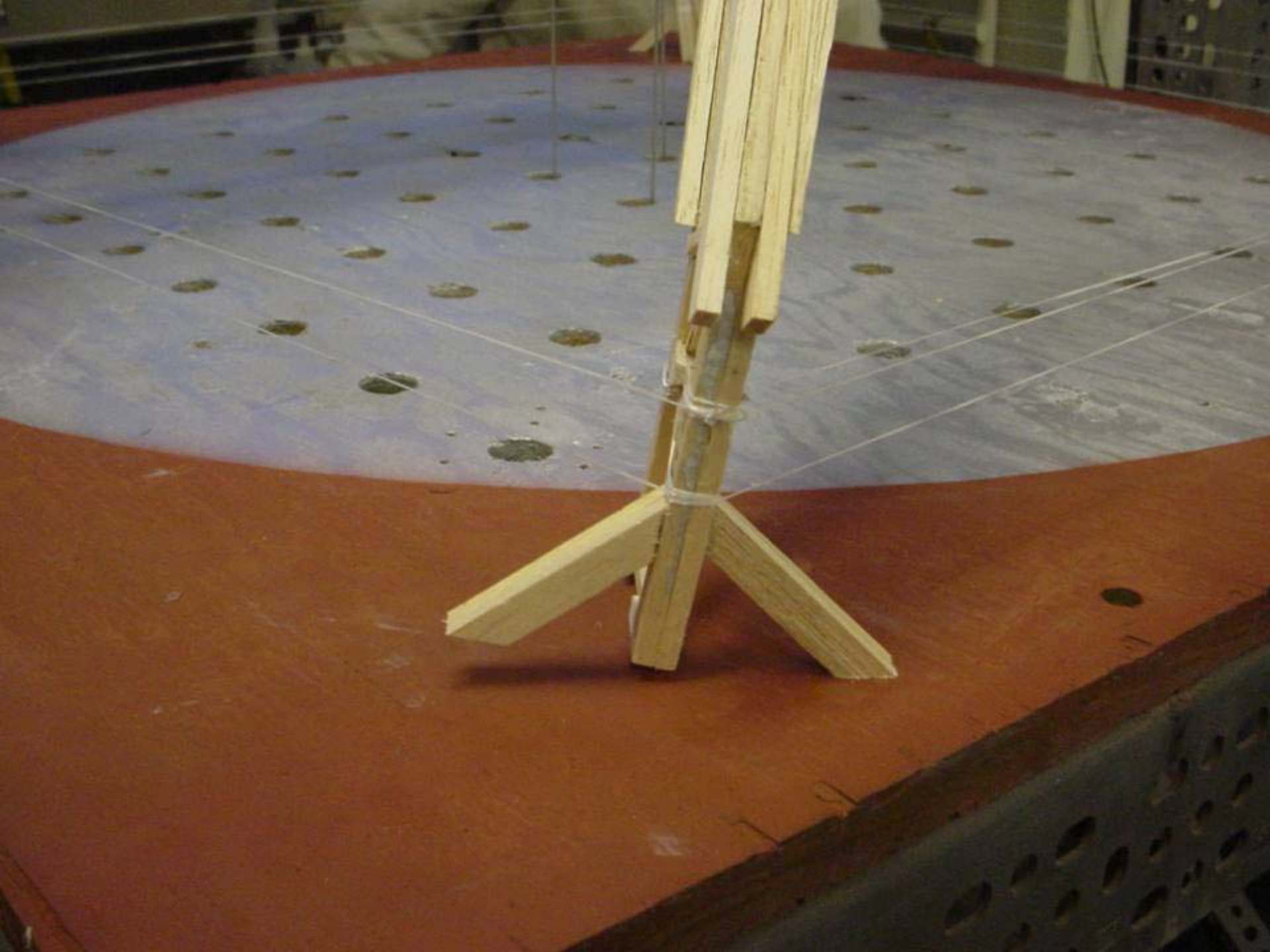


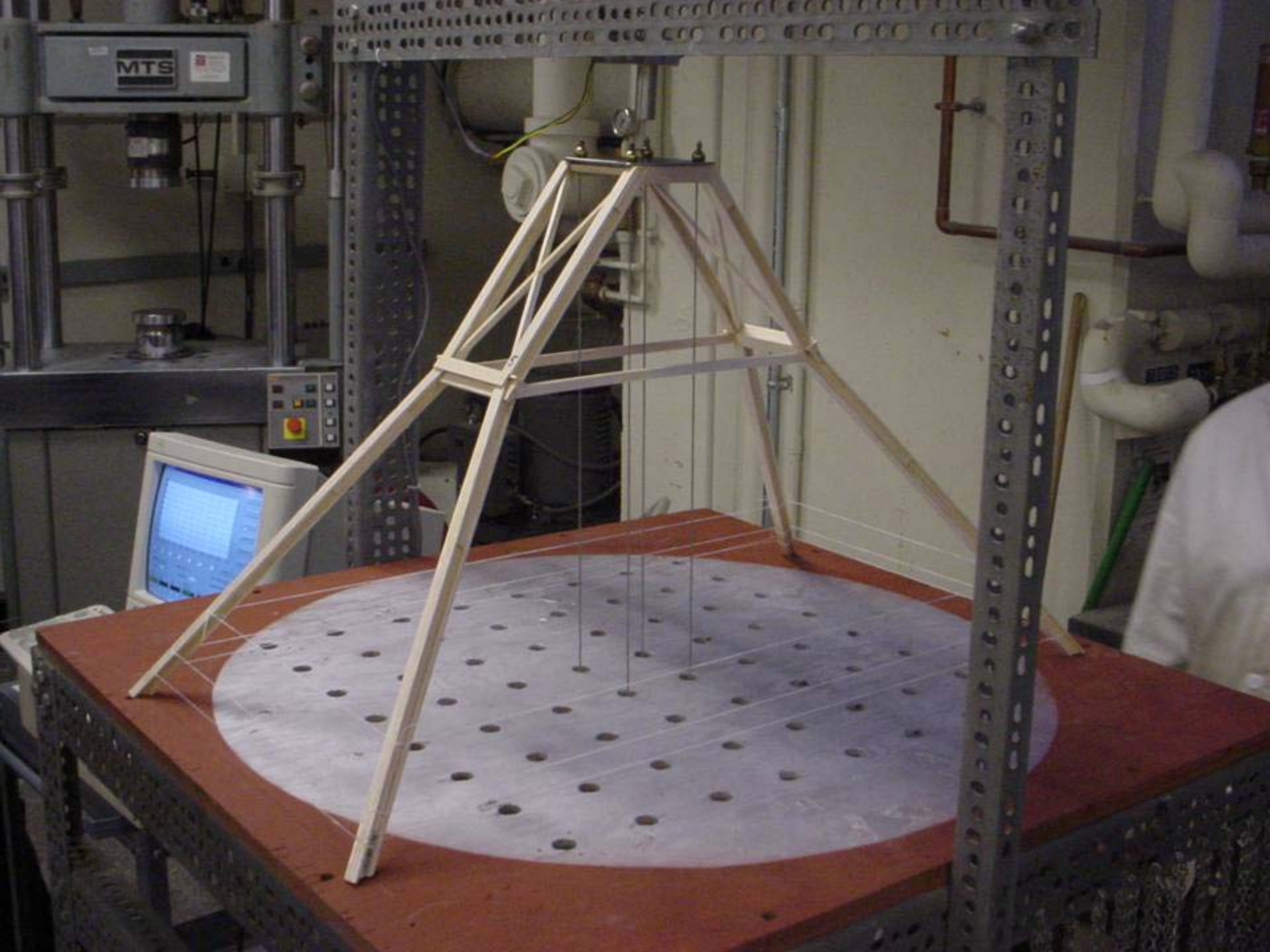


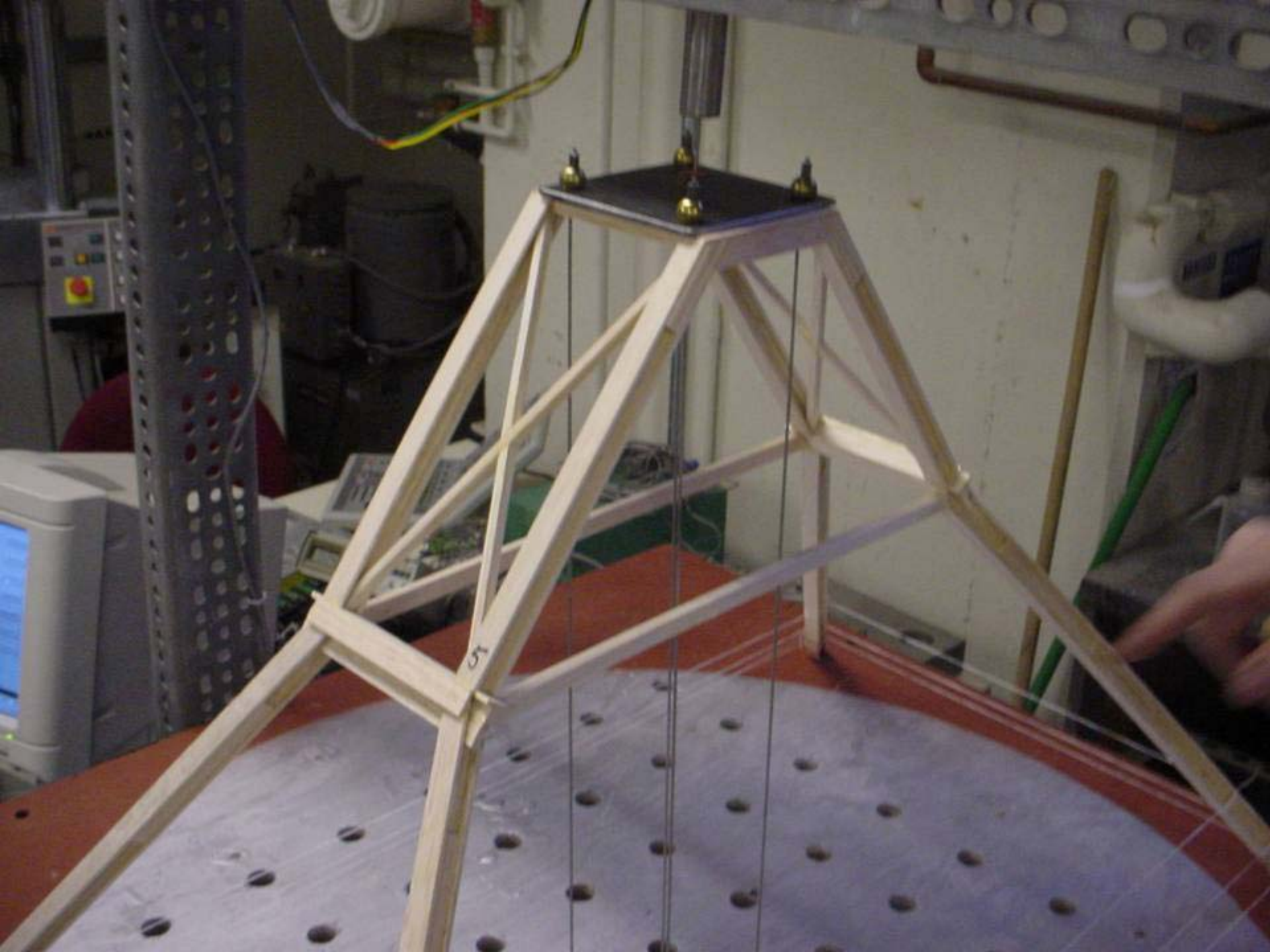


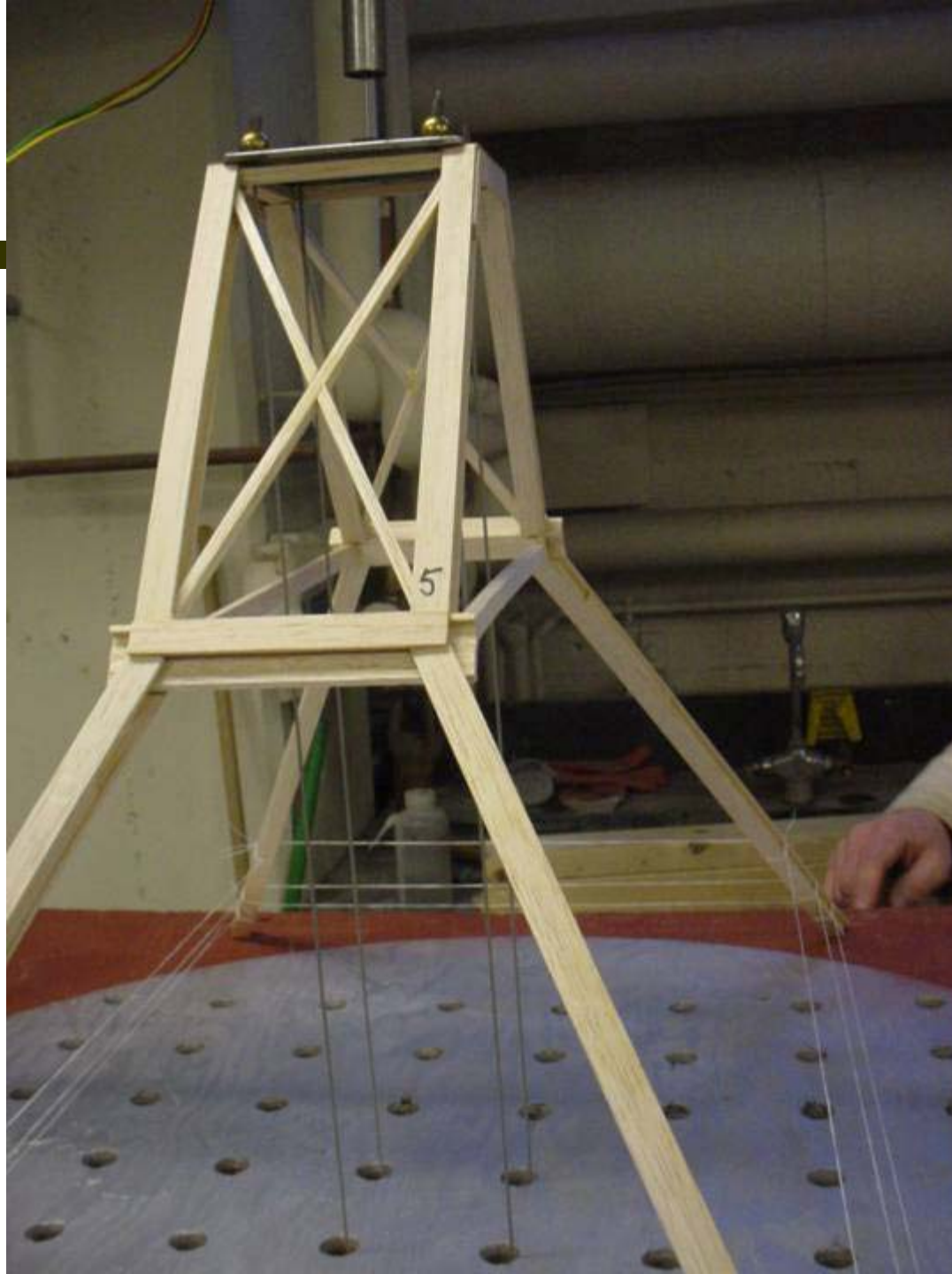


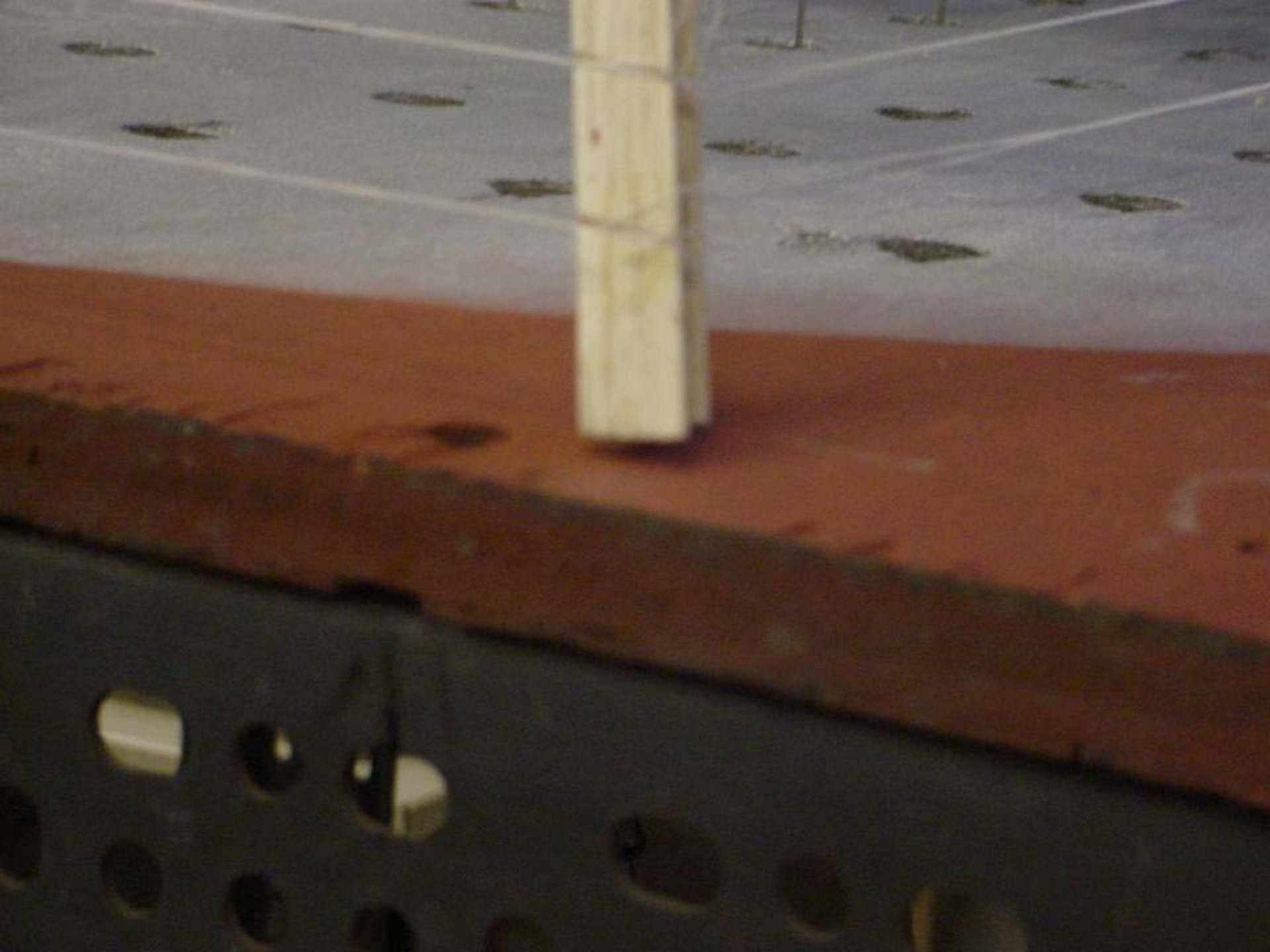


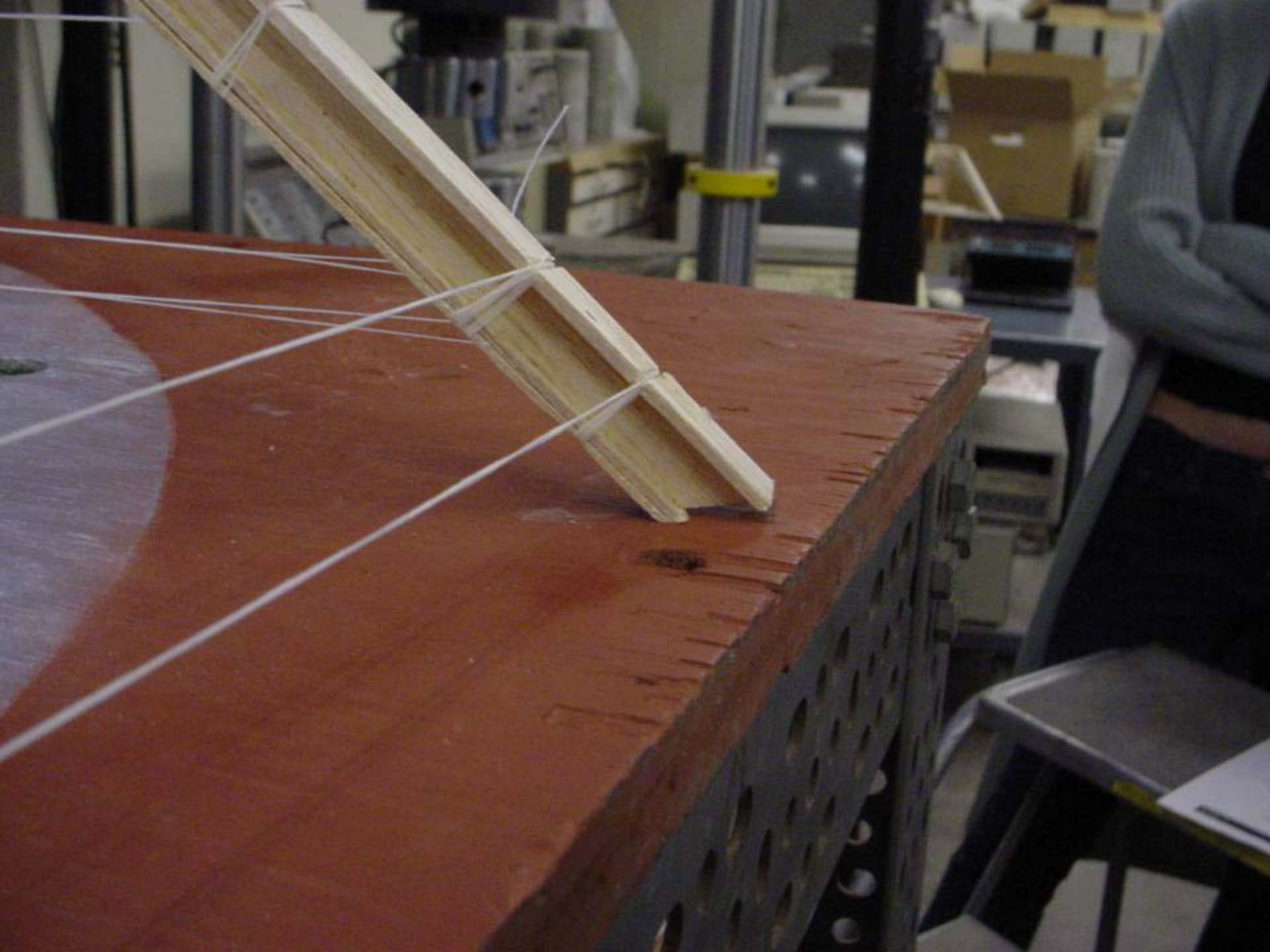


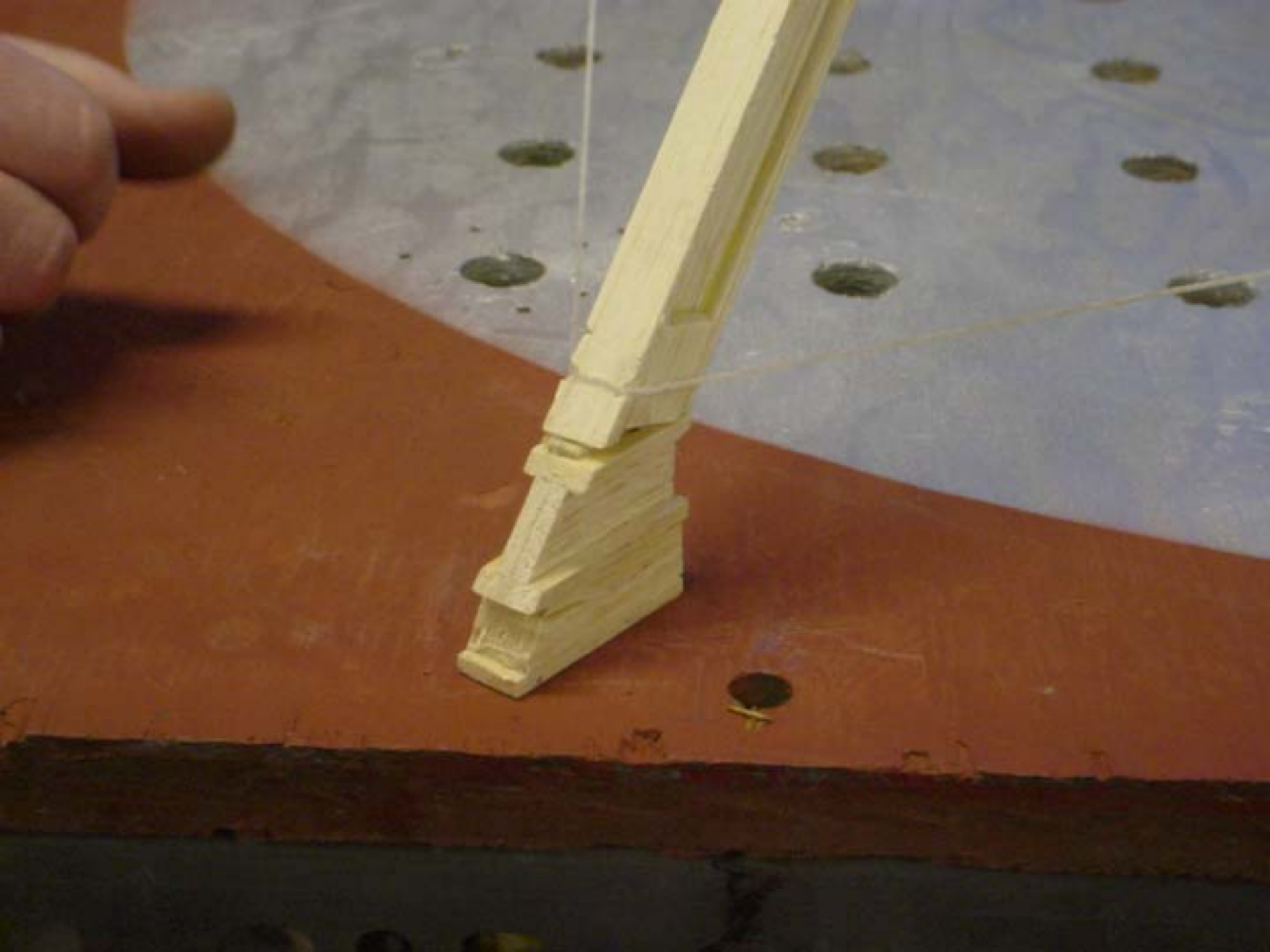


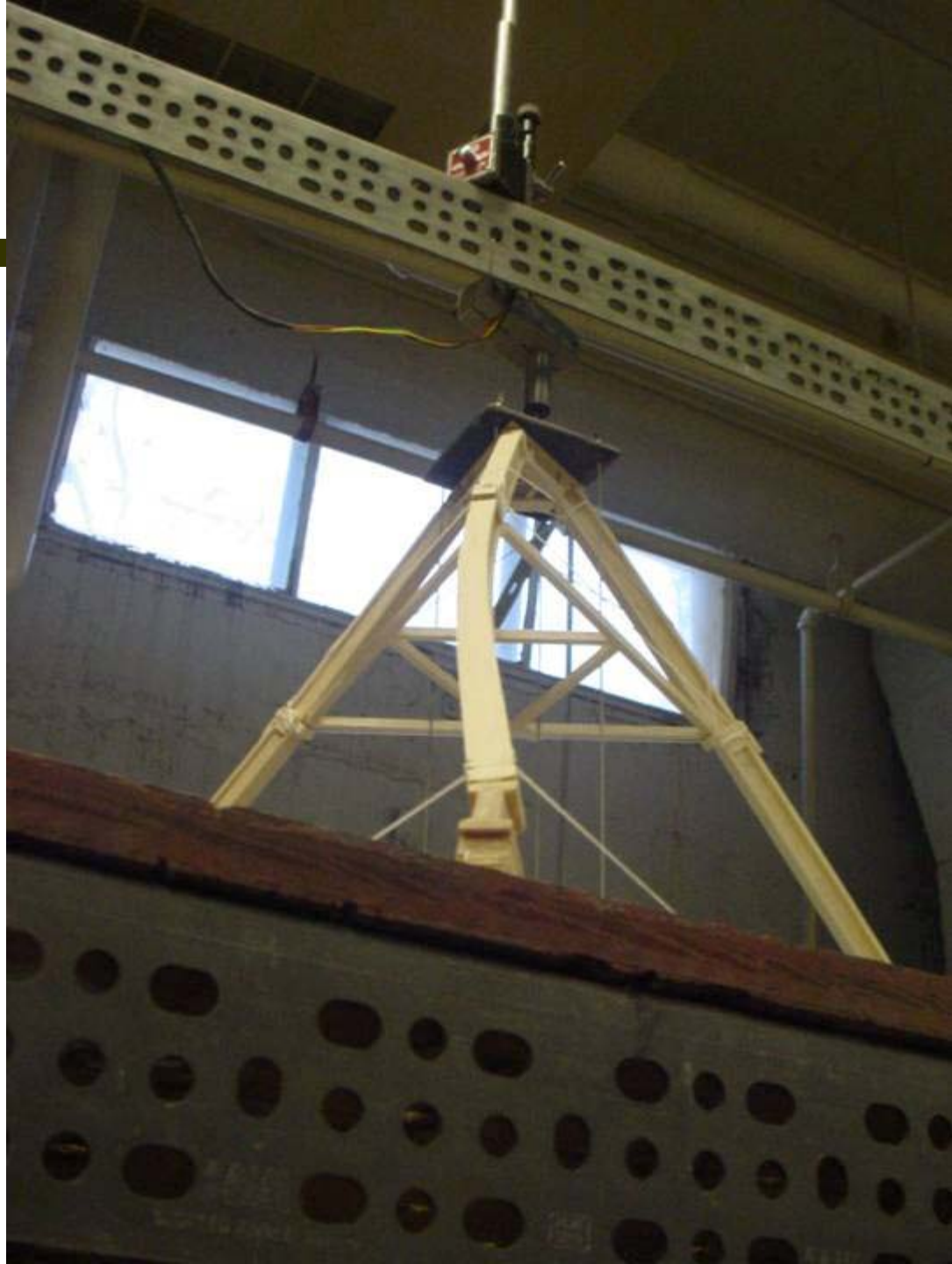






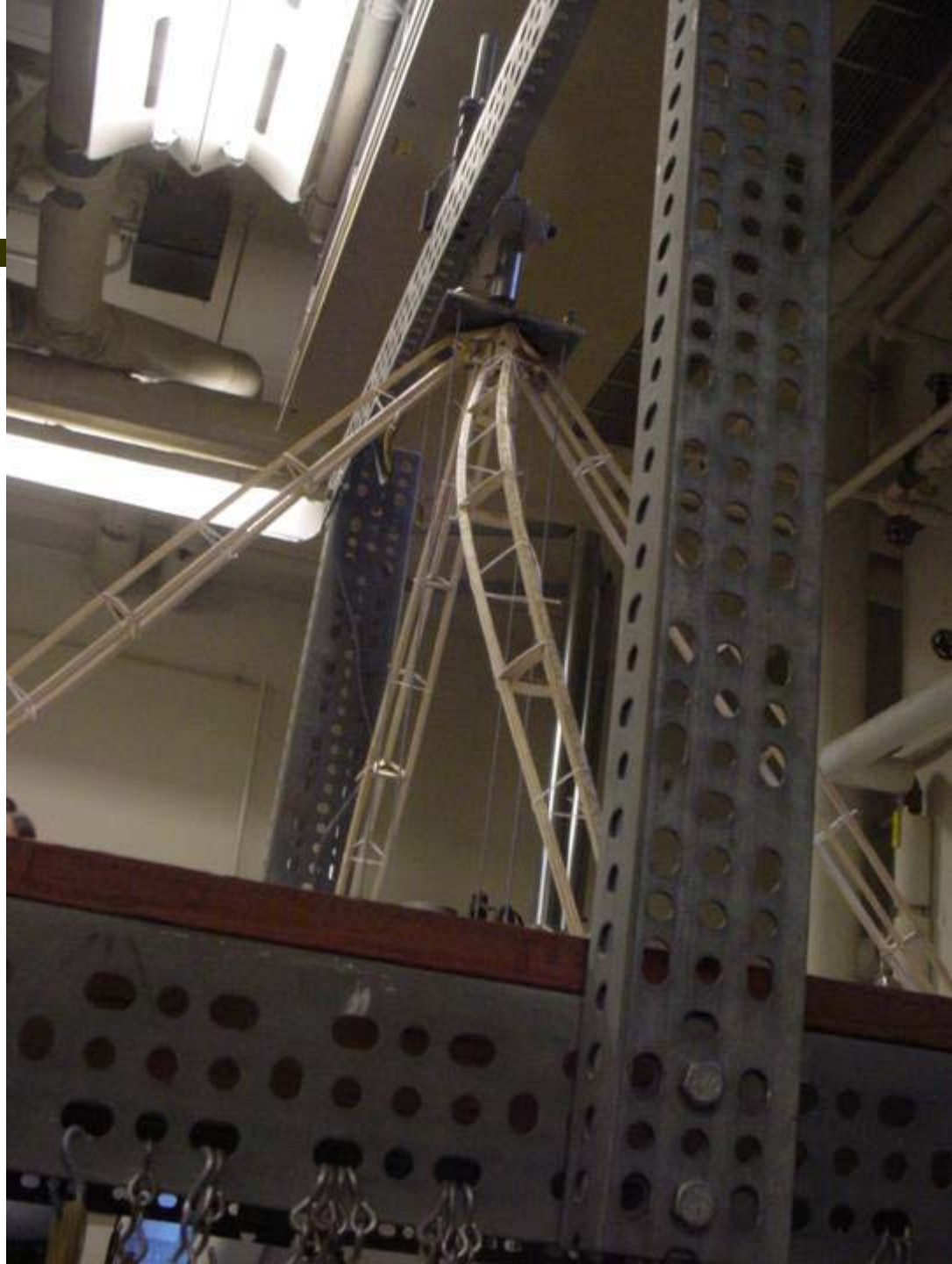






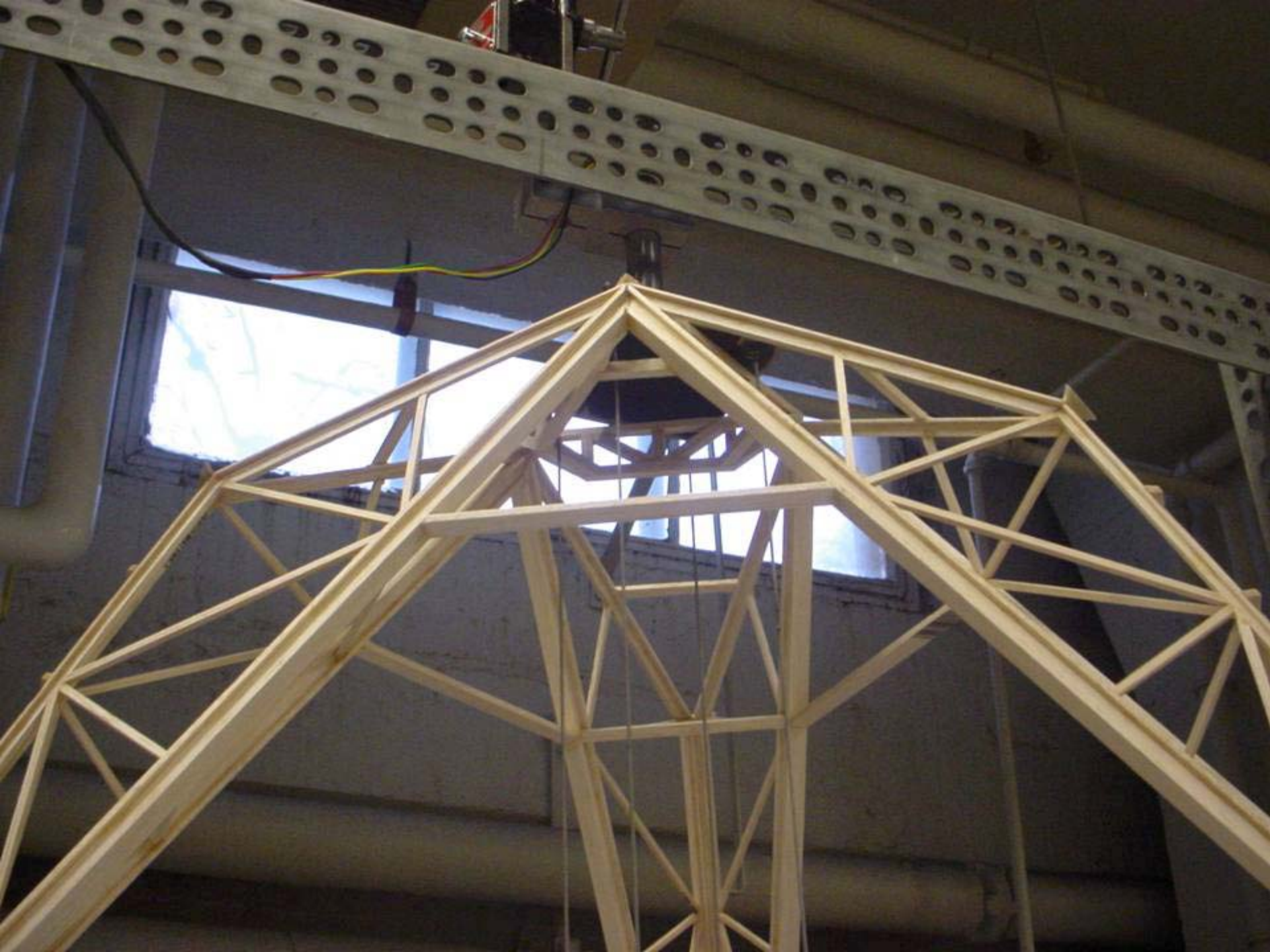


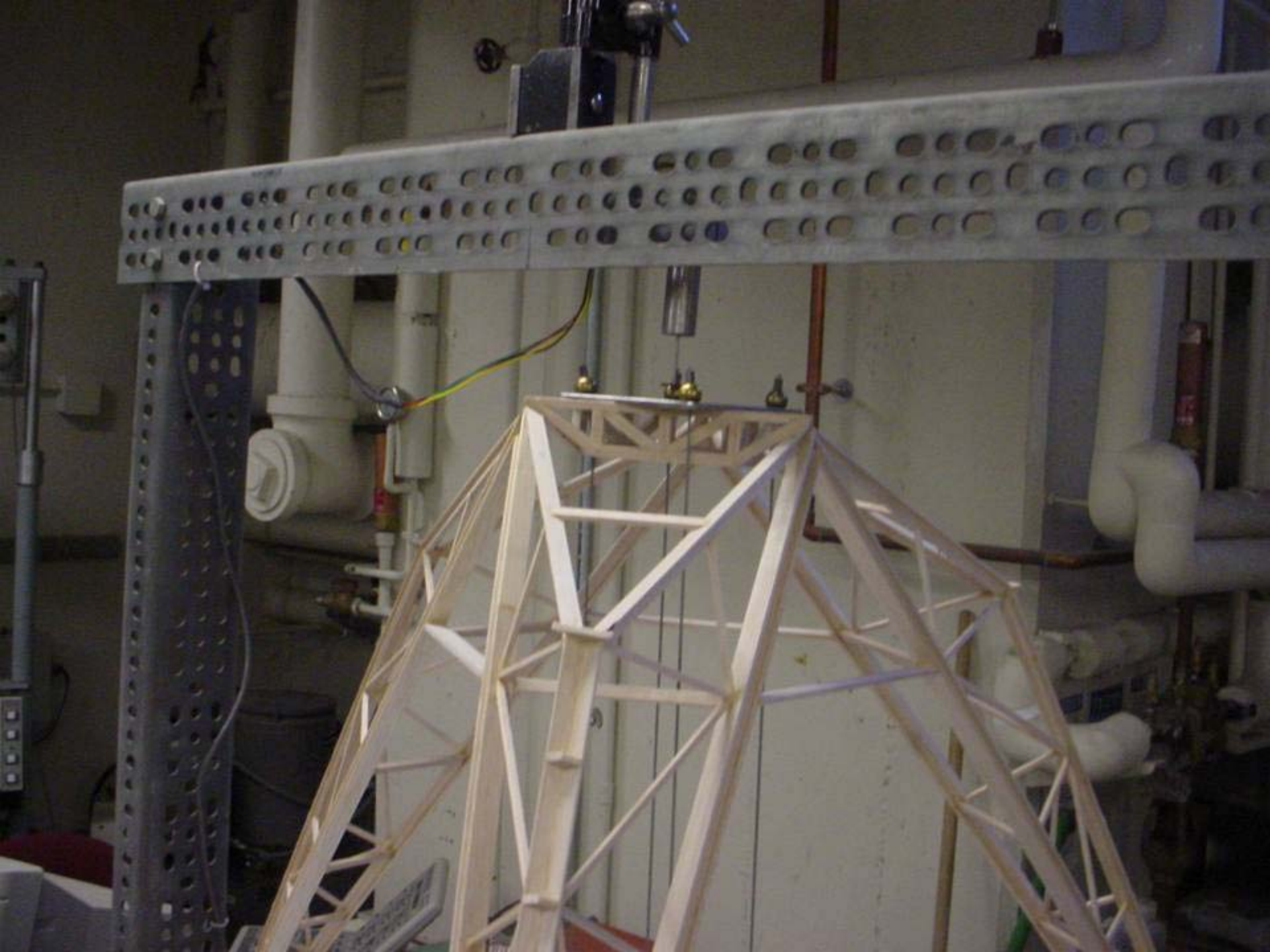


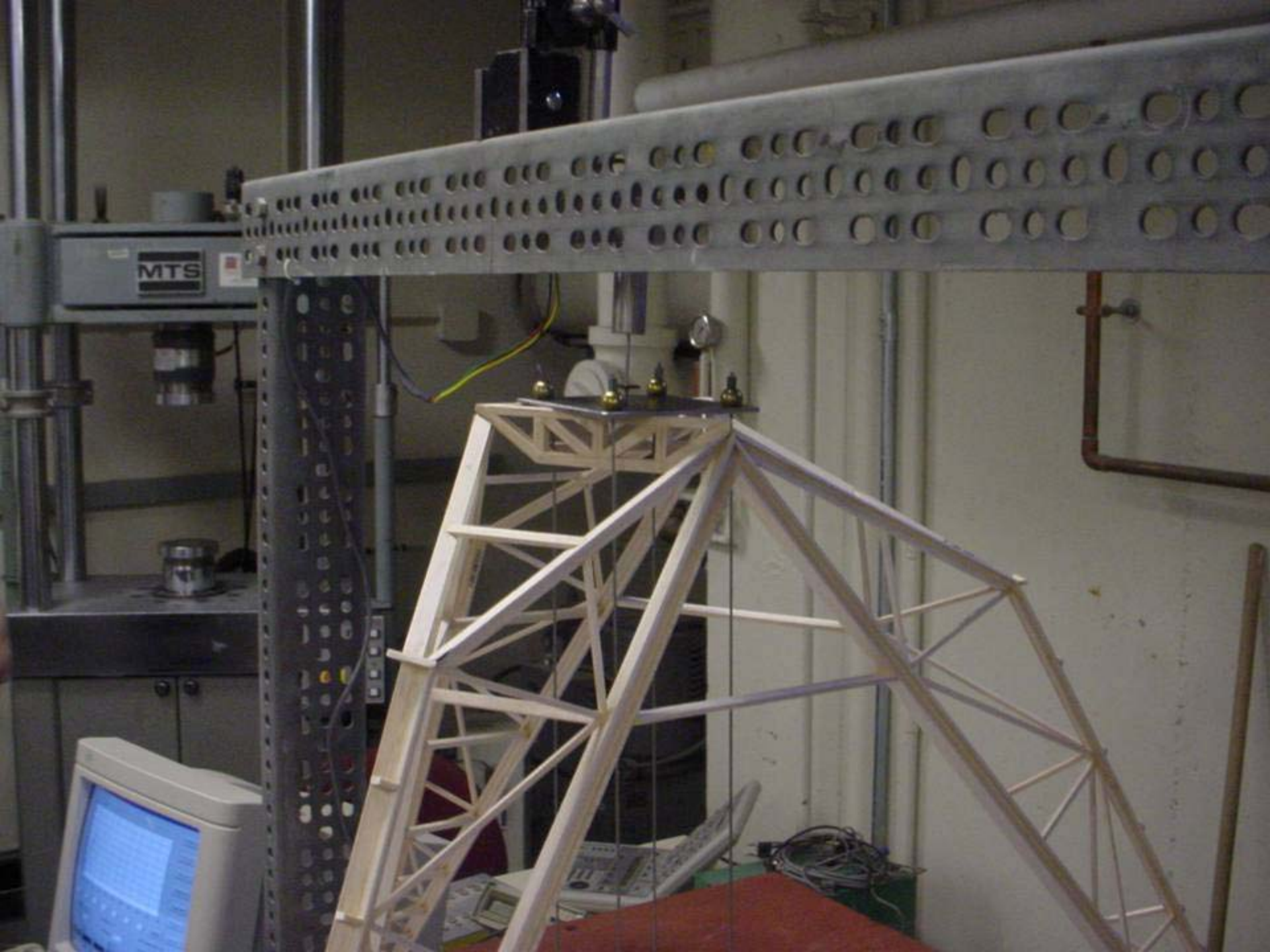




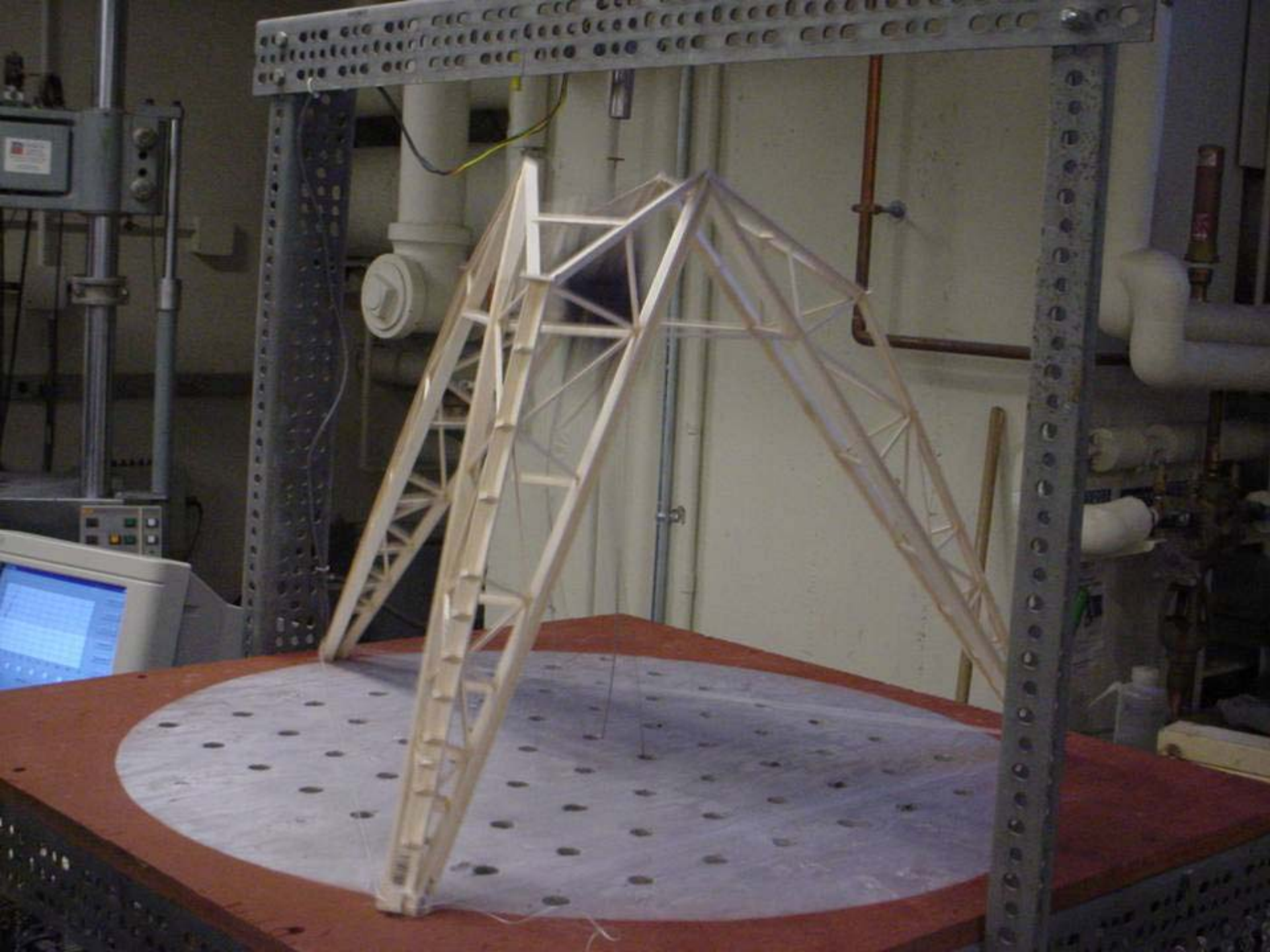


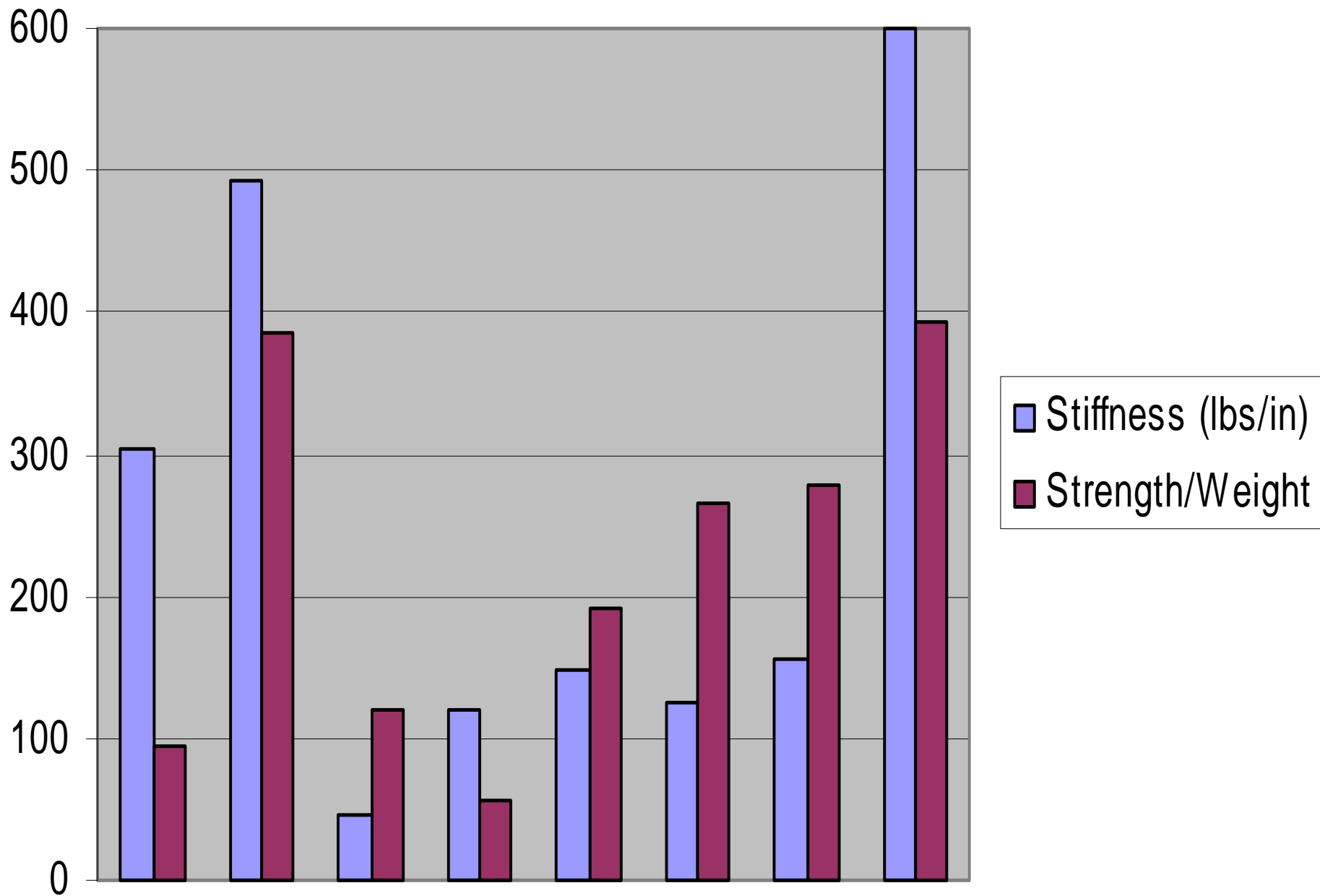




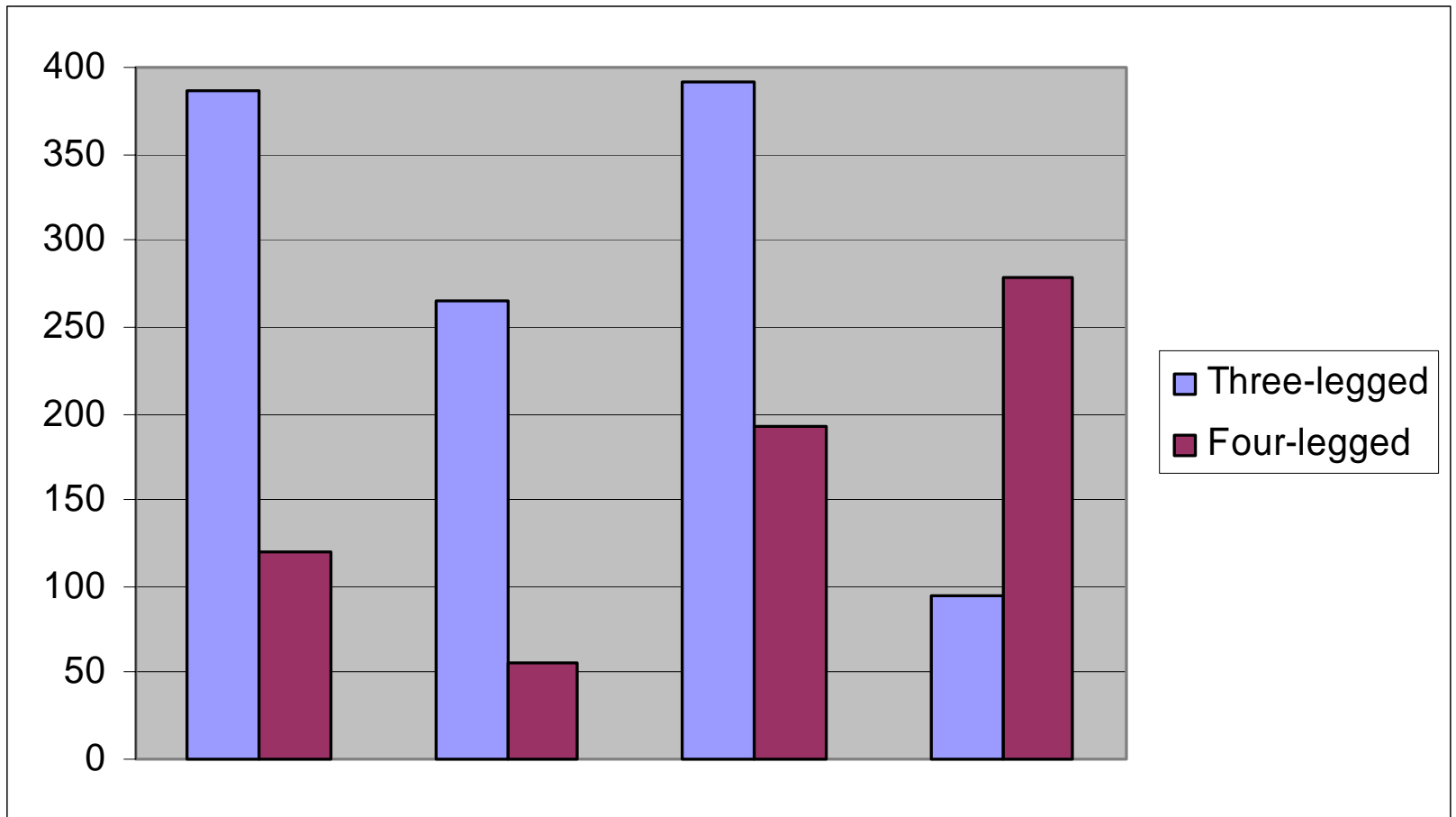




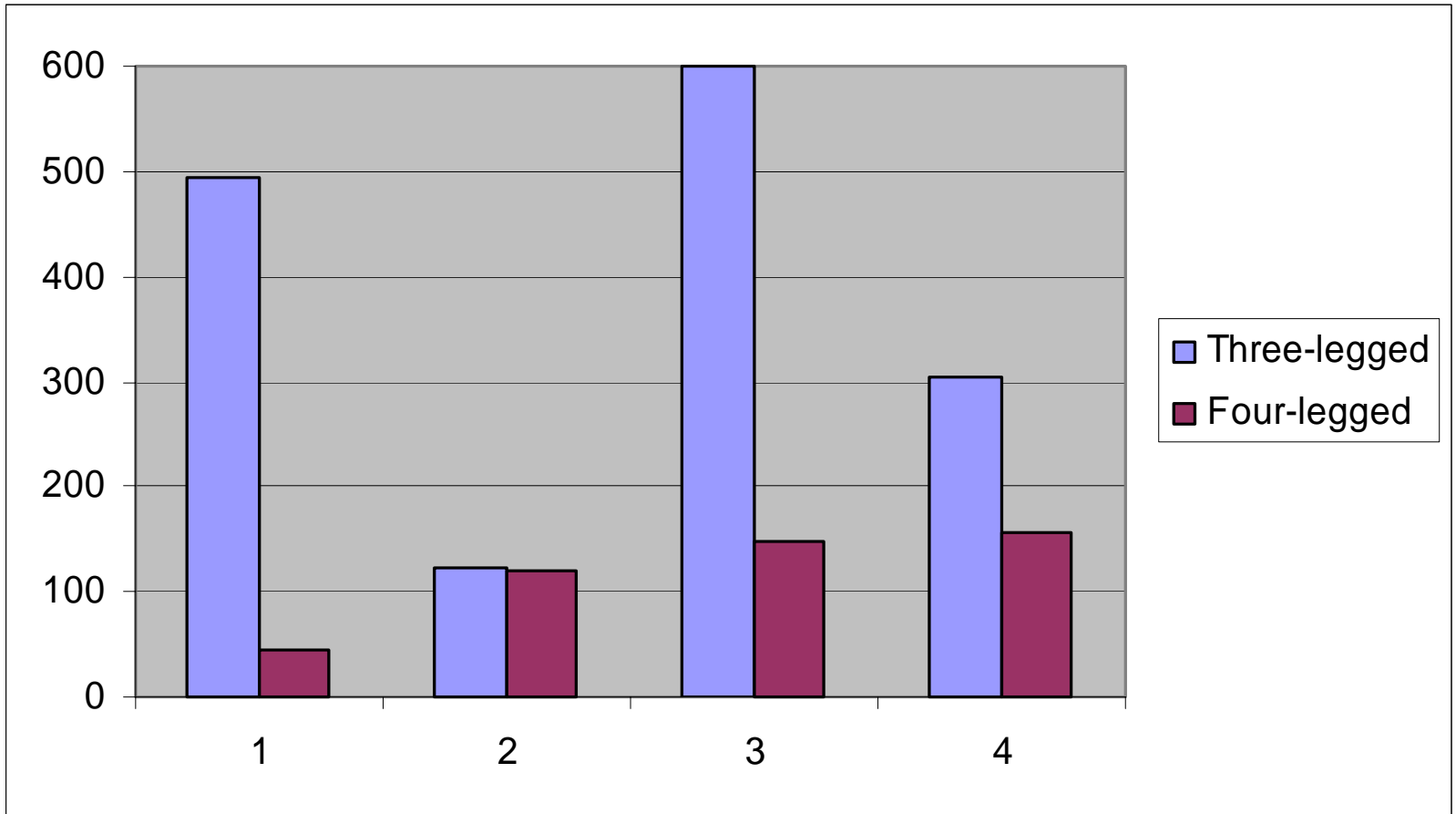




Strength to Weight Ratio

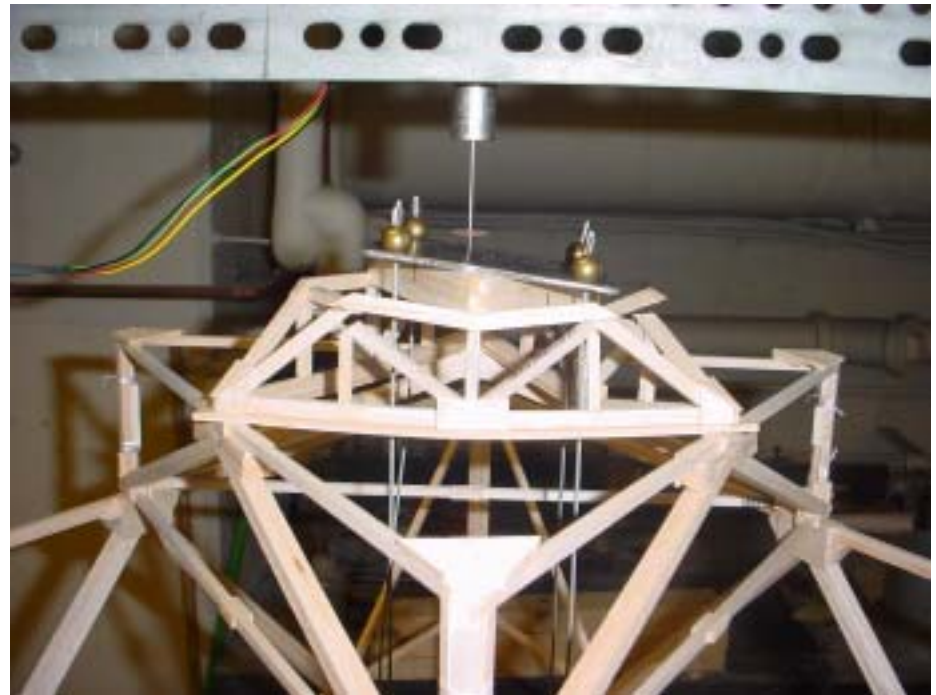


Stiffness (slope of load-displ curve)



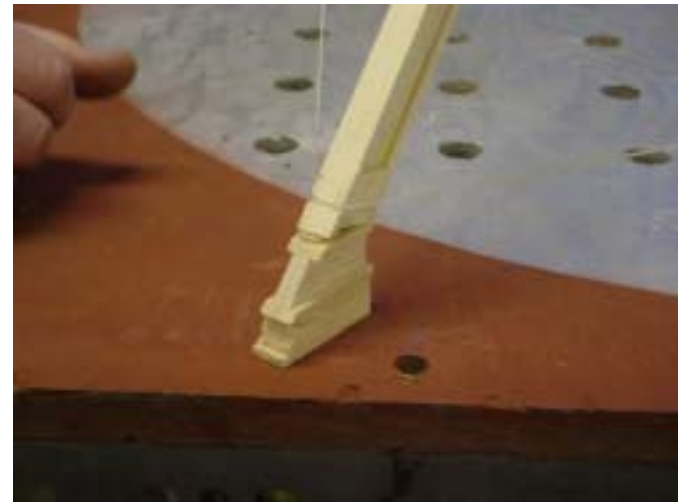
Lessons: Concept

- **Systems with many unknowns are difficult to predict**
- **Structures which are simple and clear in their load paths are easier to design**



Lessons: Construction

- **Details are critical**
- **Repetition helps**
- **Small differences add up to big differences**
- **3D joints are not easy!**



Lessons: Buckling

- **Failure occurred significantly below calculated values:**
 - **Sensitive to imperfections**
 - **Support conditions**
 - **Insufficient bracing**
 - **Lower load in post-buckled state (not a ductile failure)**
 - **Effective buckling length was typically much *longer* than assumed**

